THE CORPORATION OF THE TOWNSHIP OF ESSA COMMITTEE OF THE WHOLE MEETING WEDNESDAY, JUNE 29, 2022 6:00 p.m.

AGENDA

Members of the public wishing to attend can do so by attending in person to the Council Chambers located in the Administration Centre at 5786 County Road 21, Utopia.

1. OPENING OF MEETING BY THE MAYOR

2. DISCLOSURE OF PECUNIARY INTEREST

3. DELEGATIONS / PRESENTATIONS / PUBLIC MEETINGS

a. Cheque Presentation Re: Essa Challenge Golf Tournament Royal Victoria Hospital Foundation Stevenson Memorial Hospital Foundation Matthews House Hospice

Addedb.Nottawasaga OPP Presentation – PC Joseph ChamberlainRe: Christopher Hooyman, Essa Resident

p. 1 **c. Delegation Re: Brookfield Subdivision Agreement** Catherine Pan, Senior Development Manager – Brookfield David Murphy, Vice President of Land - Brookfield

Separate d. Presentation

Attachment Re: Draft Angus Infrastructure Master Plan

<u>Recommendation:</u> Be it resolved that the draft of the Angus Infrastructure Master Plan, as presented by the Manager of Public Works, be received; and That Council encourage the public to attend the Public Information session scheduled to be held at the Angus Recreation Centre on Thursday July 14, 2022, between 2:00 p.m. – 8:00 p.m., so that they can provide Staff with input on servicing options in the subject Plan; and

That the Manager of Public Works be directed to provide Council with a Staff Report for their consideration at a future meeting of Council.

STAFF REPORTS

4. PLANNING AND DEVELOPMENT

5. PARKS AND RECREATION / COMMUNITY SERVICES

6. FIRE AND EMERGENCY SERVICES

7. PUBLIC WORKS

p. 3 a. Staff Report PW011-22 submitted by the Manager of Public Works, re: 2022 Engineering Standards Draft.

<u>Recommendation</u>: Be it resolved that Staff Report PW011-22 be received; and That Council adopt the attached 2022 Engineering Design Standards Report to replace the current 1989 Engineering Standards.

8. FINANCE

p. 114 a. Reduction of Securities – Marshall (Brookfield) Subdivision.

<u>Recommendation</u>: Be it resolved that Council approve a reduction in securities (External Works Agreement) relating to Marshall (Brookfield) Subdivision, as recommended by Ainley & Associates Limited as follows:

| Current Securities Held by Township of Essa: | \$4,061,939.86 |
|--|----------------|
| Security to be Released: | \$2,082,046.27 |
| Securities to be Retained by Township of Essa: | \$1,979,893.59 |

And,

That this approval is conditional upon the Developer providing the municipality with a Statutory Declaration indicating that all accounts have been paid in full, including all of the Township's legal and engineering costs.

9. CLERKS / BY-LAW ENFORCEMENT / IT

p. 118 a. Report submitted by Township of Essa's Integrity Commissioner, re: Integrity Commissioner's Annual Report – Spring 2022.

<u>Recommendation</u>: Be it resolved that the "Annual Report – Spring 2022", as submitted by Principles Integrity, be received, and that the Report be made available to the public by posting on the Township's website; and That staff be directed to schedule Principles Integrity to attend a future Council

orientation session in order to provide Council with education and training on matters pertaining to the Township's ethical framework, following the 2022 municipal election.

10. CHIEF ADMINISTRATIVE OFFICER (C.A.O.)

p. 125 a. Staff Report CAO022- 22 submitted by the Chief Administrative Officer, re: 152 Greenwood Drive – Rezoning of Land to Allow for a School.

<u>Recommendation</u>: That Staff Report CAO22-22 be received; and That the CAO be directed to collect further information to use, to present to Council, to satisfy the comments and/or concerns of the public; and That a further report be presented to Council at a future date.

11. OTHER BUSINESS

12. ADJOURNMENT

<u>Recommendation</u>: Be it resolved that this meeting of Committee of the Whole of the Township of Essa adjourn at _____ p.m., to meet again on the 7th day of September, 2022 at 6:00 p.m.

Brookfield Properties

Date: June 22, 2022

Clerk's Office Township of Essa 5786 Simcoe County Rd 21 Utopia, ON L0M 1T0

Speaking Notes for June 29th Township of Essa Council Meeting Item: Subdivision Agreement – Baxter – Brookfield Residential

The following represents speaking notes for a delegation to be delivered on June 29th to the Township of Essa's Town Council.

- Brookfield has been working towards developing housing and amenities in the Baxter community since we purchased the property in 2017.
- To date, we have constructed a stormwater management pond, funded the upgrade of Baxter's Water Booster Station, made servicing and road improvements along Denney Drive, serviced our first phase of construction, completed the amended Environmental Assessment process for the wastewater treatment facility, and built two model homes.
- We are at an important point in the development process where we feel the release of building permits is appropriate.
- We are looking forward to the timely execution of a Subdivision Agreement with the Township and to delivering new homes to the Baxter community.
- We wish to thank Township staff and their consultants for all of the time and effort they have put into the development thus far.
- As the item is discussed, we are available to answer questions from Council.

Yours truly,

Catherine Pan Brookfield Properties catherine.pan@brookfieldpropertiesdevelopment.com

Brookfield Properties

3381 Steeles Avenue East, Suite 100, Toronto, ON, M2H 3S7 T +1 905 477-5711 F +1 416 492 5870 brookfieldproperties.com

| Description | Status | Timing / Completion |
|---|------------------------|---|
| Water Supply Agreement with Essa | In Place | 2018 |
| 2 Model Homes on Denney Drive | Complete | 2019 |
| Stormwater Outfall | Complete | Winter 2020 |
| Stormwater Management Facility | Substantially Complete | Winter 2020 |
| Denney Drive Reconstruction | Substantially Complete | Fall 2021. |
| Phase 1 Servicing to Base Asphalt | Substantially Complete | Fall 2021 |
| WWTP EA Amendment | Complete | Spring 2022 |
| WWTP Detail Design | Underway | Anticipate 40% submission to Township early July 2022 |
| Phase 1 Utilities | Underway | Anticipate to be late June / early July 2022 |
| Denney Drive Streetlighting | Tendering | Anticipate August / September 2022 construction |
| IPS at Denney Drive / Murphy Road | Tendering | Anticipate August 2022 construction |
| IPS at Denney Drive / Felhazi Trail | Tendering | Anticipate August 2022 construction |
| Construction of 3 Models Included in | | |
| Pre-Servicing Agreement | Underway | Re-commence construction late June 2022 |
| Subdivision Agreement | Being Drafted | Anticipate July 2022 Execution |
| Construction of Additional Models on | | |
| Denney Drive | Applied for Permits | Following Subdivision Agreement Execution |
| Plan Registration | N/A | Anticipate late July / early August Registration |
| Home Construction | Applied for Permits | Anticipate starts August 2022 |
| Water Booster Station Construction | Under Construction | Anticipate Completion October 2022 |
| Phase 2 Servicing | N/A | Fall 2022 / Spring 2023 |
| WWTP Construction | Ongoing | Anticipate start Spring 2023, completion Fall 2023 |
| 1st Home Occupancies Phase 1 | N/A | Anticipate March 2023 |



TOWNSHIP OF ESSA STAFF REPORT

| STAFF REPORT NO.: | PW011-22 |
|-------------------|---|
| DATE: | June 29 , 2022 |
| то: | Committee of the Whole |
| FROM: | Michael Mikael, P.Eng – Manager of Public Works |
| SUBJECT: | 2022 Engineering Standards Draft |

RECOMMENDATION

That Staff Report PW021-21 be received; and

That Council adopt the attached 2022 Engineering Design Standards Report to replace the current 1989 Engineering Standards.

BACKGROUND

The Township Engineering Design Standards were created back in 1989 and the standards have not been updated since that date. The entire document was outdated and does not represent any of the current industry (engineering) standards.

Council approved **\$50,000** (back in 2021) to create and update the Township Engineering Design Standards. The total cost to date remains under \$28,000 (excluding applicable tax).

The report has been circulated to all department heads for comments.

COMMENTS AND CONSIDERATIONS

The attached report is a living document and will be updated internally by staff on a yearly basis based on the future challenges and concerns.

The attached report addresses many of the concerns which previously caused issues/concerns, including:

- General information and subdivision design submissions package in a specific, phased approach including traffic calming management, pedestrian & pathway circulation plans, specifics for traffic impact assessments, visitor parking requirements, etc
- Climate change parameters as currently used by surrounding municipalities (represents a 15% increase in intensity over the rain gauge data to account for climate change)
- Requirement for computer models for water & sewer in relation to the Infrastructure Master Plan
- Driveway/ entrance design requirements
- New standards for cul-de-sacs to ensure enough turning radius for winter operations/onsite snow storage ("R20" requirement) 2

- Increased sidewalk width to 1.8 m
- Parkland and drainage requirements
- New storm/underground requirement for new development if drainage cannot be contained within the property limits to limit liability exposure with respect to sidewalks and slipping incidents

SUMMARY/OPTIONS

Council may:

- 1. Take no action and continue to use the existing document.
- 2. Adopt the 2022 Engineering Design Standards (attachment no.1) to replace the current 1989 Engineering Standards.
- 3. Direct Staff in another course of action such as continued circulation for further review.

CONCLUSION

Staff recommends that **Option 2** be approved.

Respectfully submitted,

Michael Mikael, P.Eng Manager of Public Works

Reviewed by,

Bally

Colleen Healey-Dowdall, Chief Administrative Officer

Attachment no.1: 2022 Engineering Design Standards (Consolidated)



Engineering Design Standards

&

Specifications Manual

June, 2022

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Appendices

Appendix A: Revision Information

Appendix B: Approved Products List

Appendix C: Traffic Impact Assessment Guidelines

Appendix D: Standard Drawings

1.1 INTRODUCTION

1

The Township of Essa Engineering Design Standards and Specifications Manual, including the Essa Standard Drawings ("the Manual with Standard Drawings") is intended as a guideline to:

- i. provide a good engineering basis for subdivision design;
- ii. establish a uniform criterion of minimum standards; and,
- iii. improve processing of subdivision plans and agreements in the Township of Essa.

Technological or economical changes which improve or maintain the quality of the design will be considered at the Township's discretion.

The Manual with Standard Drawings is considered a living document subject to regular review such that revisions and updates will be made from time to time at the Township's discretion. It is the responsibility of the Developer and / or Consulting Engineer to obtain and use the latest version available at the time of subdivision design.

It is understood that the Manual with Standard Drawings may be referred to as a schedule in a subdivision agreement and that the current revision of the standards are then considered to be part of the agreement.

1.2 CONSULTANT / DESIGN ENGINEER'S ROLE

The design and all required engineering drawings and associated reports must be signed and sealed by a suitably qualified Professional Engineer, licensed to practice in the Province of Ontario.

1.3 SUPPORTING STUDIES AND REPORTS

The following studies and reports may be required to support the subdivision application prior to draft plan approval. It should be noted that the information provided is the minimum requirement and the studies and reports may need to include further details depending upon site specific conditions. Additional reports or studies may be required at the discretion of the Township.

1.3.1 Functional Servicing Report

A Functional Servicing Report (FSR) shall be prepared in support of the subdivision development application for draft plan approval which addresses preliminary servicing requirements sufficient to confirm the viability of servicing the development and includes, at a minimum, the following:

i. Confirmation of adequate municipal system water and wastewater treatment capacities;

ii. Storm sewer, sanitary sewer and watermain pipe network sizing calculations and plan layout or services internal to the subdivision, showing all proposed connections to the municipal system.

1.3.2 Geotechnical Report

The Geotechnical Report shall examine and confirm subsurface conditions including soil type(s) and stratification, groundwater levels, depth to bedrock, soil bearing capacity, permeability, conductivity, and presence of any contaminates.

The report shall:

- Address the adequacy of the Township's standard minimum pavement structure and sub-drainage system or recommend a higher standard of design if conditions warrant;
- ii. Assess the suitability of native soils for trench backfill and building foundation construction. It shall also address requirements for imported fill as well as construction methods for the use of both native and imported materials;
- iii. Discuss the requirements for pipe bedding as well as engineered fill where necessary for building construction;
- iv. Be accompanied by a scaled drawing showing test pit and/or borehole locations complete with a detailed test pit and/or borehole log;
- v. Be signed and sealed by a Professional Engineer licensed to practice in the Province of Ontario.

1.3.3 Hydrogeological Assessment

Hydrogeological studies for proposed projects must be conducted by a qualified Professional Engineer and/or Geoscientist to characterize the groundwater regime from a site specific and regional perspective in order to address issues related to the following aspects (as applicable):

- i. Impacts to existing well water supplies within the project influence area;
- ii. Soil permeability and associated properties where the design of septic systems are concerned;
- iii. Water balance;
- iv. Groundwater impact assessment to area aquifers from construction activities and/or discharge of waste or wastewater;
- v. Test wells and associated testing in accordance with MECP D-5-5 guidelines to address water taking impact sustainability;
- vi. Impacts to nearby surface water bodies; and
- vii. Specific technical review of well field or wellhead protection areas.

For general road and servicing construction, the report should consider depth of services and identify likely areas where dewatering will be required, the rate of dewatering, requirements for dewatering, permits and the strategy to achieve the required groundwater levels. If dewatering is necessary, the report should contain sufficient data on groundwater quality and temperature of the receiving watercourse.

Engineering Design Standards & Specifications Manual

1.3.4 Stormwater Management Report

The Stormwater Management Report is to provide details and supporting calculations associated with the design of the minor and major drainage systems as well as the required stormwater management measures in accordance with criteria of MECP, the Conservation Authority and this document.

At a minimum, the following details must be included in the Report:

- i. Background Information;
- ii. Stormwater Management criteria to be met;
- iii. Stormwater Management Strategy including a review of feasible Low Impact Development measures;
- Pre-Development and Post-Development Drainage Areas; iv.
- Storm Drainage System Design major and minor conveyance systems; v.
- Design of End of Pipe Controls; vi.
- Erosion and Sediment Control; vii.
- viii. Inspection and Maintenance Requirements;
- Operation Manual outline; and, ix.
- Calculations, Tables, Figures, Modeling, and Drawings х.

1.3.5 Traffic Impact Assessment

Traffic Impact Assessment Guidelines are included in Appendix C of this document which provide a consistent approach to be followed by applicants for the preparation of a Lraffic Impact Assessment (TIA) in the context of a development application. In part, these guidelines include thresholds based on predicted traffic volumes which identify the need for a TIA in general and also as to when a Traffic Impact Study is required versus a Traffic Impact Brief.

The scope of work for each type of study is identified in the TIA guidelines in accordance with MTO, TAC and ITE guidelines and criteria as applicable.

In general, a TIA shall consider and review existing and projected traffic levels under the horizon years in the TIA Guidelines to determine requirements for traffic control devices and intersection improvements, including storage length requirements for turning lanes, based on peak traffic flows that have been generated. In addition to vehicular traffic, the report shall identify how cyclist and pedestrian needs are satisfied, including those with barrier free requirements.

The area of influence of a development should include major intersections which may not be directly adjacent to the development.

The report shall include a summary of sight lines and a comparison of existing and proposed road geometry with the MTO Geometric Design Guidelines for Ontario Roadways. For the reconstruction of existing roadways, the report should include a "Roadside Safety Review" identifying any deficiencies in road geometry, sight lines, clear zones, and roadside protection needs.

The traffic study should provide a summary of existing and proposed roadway cross-sections.

All new residential developments require provisions for off-street visitor parking facilities. The traffic study shall identify suitable locations and number of spaces required.

The traffic study should include considerations for municipal by-laws as well as the requirements of other road authorities such as the County of Simcoe and the MTO, where applicable.

1.3.6 Noise and Vibration Study

The Noise and Vibration Study shall review all potential noise and vibration sources and evaluate mitigation requirements to achieve compliance with MECP guidelines, the Township's Noise By-Law and this document.

Where noise attenuation fences and / or berms are proposed, the report should detail the locations and height required to attain acceptable noise levels, including details which address grading and drainage impacts.

For railway traffic, the report should also identify foundation requirements to attenuate vibration.

The report should consider the construction activities typically expected and provide guidance on development of a preconstruction survey of adjacent buildings and structures.

A post construction survey or report may be required by the Township to confirm that acceptable noise levels have been achieved.

1.3.7 Environmental Impact Study

The Environmental Impact Study shall be prepared by a qualified professional and will assess any potential impact of the proposed project upon the natural environment including, but not limited to wetlands, woodlots and natural habitats for threatened and/or endangered species. The report shall also characterize the impact and make recommendations for mitigation, if necessary. Additional details and requirements can be found in the Township's Official Plan.

1.3.8 Environmental Site Assessment (ESA)

For any lands that are to be dedicated to, or purchased by the Township, an Environmental Site Assessment may need to be completed.

If required, a Phase I ESA shall be undertaken by a "qualified person" as defined in O. Reg. 153/04 (or the latest revision thereof) and a report shall be completed in accordance with the requirements set out in the same regulation as well as the guidelines published by the Canadian Standards Association (CSA).

The report shall include, but not be limited to:

- i. Confirmation of Insurance coverage;
- ii. Certification that the person undertaking the assessment and completing the report is a "qualified person";

- iii. The "qualified person's" opinion as to whether a Phase II ESA is warranted based on the findings of the Phase I ESA;
- iv. If the "qualified person's" opinion is that a Phase II ESA is not required, the report shall expressly include a statement that "in his/her opinion and based on the findings of the Phase I ESA, a Phase II ESA is not necessary"; and
- v. Confirmation that the report may be relied upon by the Township in making the decision to accept ownership of the property.

1.3.9 Archaeological Report

The Archaeological Report shall assess any significant historical features on the site and recommend a mitigation plan if necessary. The study shall follow the Ontario Ministry of Culture guidelines for determining the archaeological potential of the area impacted by construction. As a minimum, the report shall follow the requirements of a Phase 1 Archaeological survey and proceed with Phase 2 and 3, if necessary.

The Archaeological Report shall also consider "Built Heritage" and provide an inventory of any historically or culturally significant structures within the affected area. The Essa Historical Society shall be contacted during the preparation of the report and the report shall include any comments they may have on the affected area.

1.3.10 Barrier Free Considerations

All design projects including new or replacement sidewalks, paths, trails, parks etc. throughout the Township of Essa must comply with the Accessibility for Ontarians with Disabilities Act and must incorporate ways to remove barriers for the public.

1.4 ENGINEERING SUBMISSIONS

- 1.4.1 Engineering Drawings General
 In general, a complete engineering drawing set shall contain, as a minimum, the following drawings plus any others that may be necessary to address Conditions of Draft Plan Approval.
 i. Cover Sheet including key plan;
 - ii. Legal and Reference Plan of Subdivision;
 - iii. General Servicing Plans showing all structures, underground and aboveground services and utilities;
 - iv. Detail Grading Plans;
 - v. General Phasing Plan;
 - vi. Sediment and Erosion Control Plans;
 - vii. Storm Drainage Area Plans;
 - viii. Sanitary Sewer Drainage Area Plans;
 - ix. Stormwater Management Pond Detail Plans;
 - x. Park Development Plan;

- xi. Street Plan and Profile Drawings;
- xii. Landscape Planting Drawings;
- xiii. Tree Preservation Plan;
- xiv. Street Light Illumination Drawing and Electrical Design;
- xv. Composite Utility Drawings;
- xvi. Detail drawings including pertinent standards and special details.

NOTE: All drawings are to be provided with a Title Block, revision summary, benchmark, scale, legend, north arrow, "Accepted for Construction" signing blocks for Township and Township Engineer and Block for the stamp of the design engineer.

All drawings to be standard A-1 size sheets (594mm x 841mm).

All plans to include a Key Plan in top right corner indicating the location of the street within the plan of subdivision.

1.4.2 Engineering Submission Requirements

First Submission

Two complete paper sets of the following plans and documents are required for the first submission:

- i. Proposed final plan for registration;
- ii. General services plan showing aboveground and underground works;
- iii. Plan and profile drawings for watermain, sanitary sewer and roadworks;
- iv. Sanitary drainage area plan and design sheets, including external areas and contours;
- v. Storm drainage area plan and design sheets, including external areas and contours;
- vi. External watermain layout plan illustrating the existing distribution system;
- vii. Miscellaneous and special details pertaining to storm sewer, sanitary sewer or watermain design;
- viii. Grading plans;
- ix. Traffic signal drawings or drawings indicating underground works (conduits, maintenance holes, hand wells, etc.) for future traffic signal installation;
- x. Pavement marking and signing;
- xi. Cross-section drawings where roads are being widened or extended;
- xii. Storm Water Management report;
- xiii. Sewer pipe strength and bedding requirements are to be provided;
- xiv. Geotechnical report.

When these drawings, documents and calculation sheets are reviewed, comments will be returned to the consultant for revisions as necessary.

Second Submission

Provided all First Submission comments have been satisfactorily addressed as confirmed by the Township, the Applicant may submit two (2) complete ECA application packages to the for review and processing.

An electronic copy (i.e. on USB drive) of the design drawings and two complete paper sets of the following plans and documents are required for the second submission and must be sealed by a professional engineer, as follows:

- i. All drawings referred to in First Submission section, incorporating all changes required from review comments;
- ii. Letter describing changes and updates;
- iii. All revised calculation sheets, if required;
- iv. Cost estimate of Township services;
- v. Draft R-Plans and easement documents;
- vi. Submit applications, fees and documentation to respective agencies and inform the Township of any other approvals required, e.g. MTO, pipelines; NVCA; MNRF, County of Simcoe
- vii. Landscape drawings.

Interim Submission

- i. Submit two copies of only those documents, drawings, schedules and calculations that required correction. The revised documents are to be stamped and sealed by a professional engineer;
- ii. Letter describing changes and updates,
- iii. Utility coordination plans.

Final Submission / Servicing Preparations

Three complete sets of construction plans and documents are required for the final submission and are to be sealed by a professional engineer, including the following:

- i. All drawings incorporating all changes as referred to in the first, second and interim submissions;
- ii. All drawings in PDF format;
- iii. Completed Form 1 document with description of proposed watermain works and specific design drawing reference, signed by design engineer of record;
- iv. Final sewer design calculation sheets;
- v. Final cost estimate of services.

1.4.3 Construction Commencement Requirements

Prior to the commencement of construction, the Township requires confirmation that:

- i. All applicable Township securities have been paid;
- ii. Acceptable insurance is in place. All Contractors working within the right-of-way must provide proof of liability insurance, minimum \$5.0 million limit, naming the Township of Essa as additional insured;
- iii. All easement documents and deeds have been received;
- iv. All external easements have been registered prior to any work outside the limits of the development;
- v. The development agreement is executed by the Applicant and returned to the Township, together with all financial securities and any other deliverables defined in the agreement.

The Consultant shall:

- i. Provide notification to the Township a minimum of 5 days prior to the commencement of construction,
- ii. Arrange a pre-construction meeting with the contractor, consultant and Township. This meeting is required to determine schedule, approvals, main service connections and testing methods. The name and contact information of the Consultant's full-time Inspector are to be provided to the Township at the pre-construction meeting;
- iii. Supply full time inspection required to ensure that the work is performed as specified. Any changes or deviation from the plans and specifications will require clearance from the Township and/or other agencies if required;
- iv. Where concrete pressure pipe (CPP) watermains are proposed, shop drawings reviewed and signed off by the Applicant's engineer shall be supplied to the Township for review and approval prior to construction;
- v. Ensure all "As-Built" drawings and service record sheets are completed and submitted to the Township including all service ties as required;

In general, the Applicant's consultant shall be responsible to ensure the requirements in the development agreement and in this manual are carried out.

Road Occupancy Permit

All work carried out within the Township's right-of-way is subject to a Township Road Occupancy Permit. The Applicant or his contractor must complete this permit application, including 2 sets of folded approved plans and the application fee as per the latest Township Fee By-Law. All works conducted within the Township right of way must be done so in accordance with Ontario Traffic Manual Book 7, Temporary Conditions. In addition, the operation of Township roads must be maintained at all times and all cost associated with traffic control shall be at the sole expense of the Applicant.

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1.4.4 "As-Built" Drawings

Upon completion of the installation of underground services, the Township shall require "as-built drawings" before issuance of the Certificate of Substantial Completion and Acceptance for any of the works as defined in the Town's standard subdivision agreement, as well as a digital copy of the plans in PDF and AutoCAD formats.

Upon completion of the maintenance period and before issuing the Certificate of Maintenance and Final Acceptance as defined in the Town's standard subdivision agreement, two full sets of "as-built" engineering drawings are to be forwarded to the Township Engineer for review and comments. Revisions must have been made to the drawings to reflect any changes to the line and/or grade of the roadways and services, and to incorporate all the grading modifications resulting from final lot grading. All water and sewer valves, hydrants, curb stops and service connections shall be properly tied into fixed reference points.

If any revisions are required, one set of red lined drawings will be returned to the Developer's Engineer.

When all revisions and/or corrections have been made a complete set of "as-built" hard copies shall be submitted to the Township Engineers as well as digital PDF and AutoCAD drawings.

The "as-built" drawings shall include the following information:

Road System

- i. Elevation of centreline of roadway every 20 metres
- ii. Revised horizontal and vertical curve information;
- iii. Any additional information that has been required for construction after approval of engineering drawings;
- iv. Revised benchmarks located in a permanent location throughout the new development at sufficient intervals such as on fire hydrants and/or other permanent structures.

Storm System

- i. Invert elevations of all storm sewers;
 ii. Invert elevations of all storm manholes;
- iii. Revised percentages of all storm sewers along with "as-built" distances between manholes;
- iv. Revised storm sewer design sheets;
- v. Location measurements to all storm sewer connections to each individual lot. These should have swing ties from property corners or other fixed structures such as fire hydrants and manholes;
- vi. As-built elevations of road cross culvert inverts;
- vii. Any additional information that has been required for construction after approval of engineering drawings.

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Sanitary System

- i. Invert elevations of all sanitary sewers;
- ii. Revised percentages of all sanitary sewers along with "as-built" distances between manholes;
- iii. Revised sanitary sewer design sheets;
- iv. Location measurements to all sanitary sewer connections (or pressure sewer valves where applicable) to each individual
 lot. These should have swing ties from property corners or other fixed structures such as fire hydrants and manholes;
- v. Any additional information that has been required for construction after approval of the engineering drawing.

Water System

- i. Elevations of top of watermain every 30 metres;
- ii. Location measurements to all water service boxes for each individual lot. These should have swing ties from property corners, buildings or other fixed structures such as fire hydrants and manholes;
- iii. Location by measurement of tees, bends, valves and dead ends;
- iv. Any additional information that has been required for construction after approval of the engineering drawing.

Lot Grading

- i. Any major revisions to the approved lots;
- ii. Invert elevations of all swales, sufficient to confirm intermittent swale grades;
- iii. Invert elevations of all culverts;
- iv. Invert elevations of all ditches at 20 metre intervals.

2 Drafting Requirements

2.1 FORMAT AND QUALITY

All drawings are to be prepared in digital AutoCAD format, geo referenced to UTM NAD 1983, Zone 17N.

All original drawings and prints shall be neat and legible and shall be corrected for "As-Built" in the same manner. All information shall be reproducible by a white-printing or photocopy machine.

2.2 DRAWING SHEET SIZES

Drawings shall be of a consistent size of 594mm by 841mm (metric size A1)

2.2.1 Scales

Standard metric scales to be used are 1:100, 1:200, 1:250, 1:500, 1:1000 and their factors of 10. Scales shall be as follows and shown on the drawings:

- i. the key plan shall be shown on the cover sheet at a scale of 1:5000;
- ii. the General Service Plan and the Sanitary and Storm Sewer Plans shall be 1:1000;
- iii. the Lot Grading Plan and the Park Development Plan shall be 1:500, or 1:250 if required;
- iv. Plan and Profile Drawings shall be 1:500 (Horizontal) and 1:50 (Vertical).

2.3 BASIC INFORMATION

The following standards shall apply in preparation of the drawings:

- All plans shall include a north arrow in the upper right-hand quadrant. All east-west streets shall generally be drawn with the north arrow pointing to the top, all north-south streets with the north arrow generally pointing to the right, and all cul-de-sacs or other roads where this does not apply shall be drawn with the stations numbered from left to right.
- ii. All elevation data shall be referred to geodetic datum and at least one benchmark shall be shown on each plan indicating a proposed elevation.
- iii. In general, east-west streets shall have zero chainage at their westerly limits and north-south streets shall have zero chainage at their southerly limits. Chainages on a plan-profile shall increase from left to right.
- iv. All existing utilities, structures and other features such as trees and hedges shall be shown and identified using a broken line.
- The beginnings and ends of curves must be shown on plan and profile with the radius of curvature shown on the plan.
 Chainages of points of curvature shall be calculated from the final plan. The chainage elevations and names of intersecting streets shall be shown in plan and profile.

- vi. Street names shall be kept clear of the road allowance.
- vii. The drawings shall show any required off-street drainage and separate profiles shall be prepared for drainage easements.
- viii. The drawings shall clearly show the proposed profiles, road widths and cross-sections, ditches, ditch gradients, curb and gutter gradients, culvert sizes / gauges and gradients, existing and proposed services and limits of the proposed work. All detail for intersecting streets including grades must be shown for a minimum distance of 30 metres from the intersection of the intersecting street. All street lines and easements for drainage or services shall be shown. Larger scale detail may be required for congested bends and/or cul-de-sacs.
- ix. The drawings shall show the lot frontage distances and dimensions of easements and land to be dedicated to the Township.
- x. The Township's Consulting Engineer shall be consulted as to the manner of showing information not set out in these requirements.

2.3.1 Sewer Details

The standard abbreviations, sewer diameter, sewer pipe material, length, grade, manholes, inlets and connections to the sewer shall be shown on appropriate General Plans. This information plus sewer bedding, type and class of sewer pipe, manhole numbers, diameters and inverts, flow direction, grate elevations and drop structures shall be shown on Plan and Profile Drawings. Chainage of manhole locations shall be shown in profile. Service locations to be shown on plan drawing.

2.3.2 Watermain Details

The standard abbreviations, watermain diameter, length, type and class of pipe, and the valves, services, hydrants, bends and connections to the watermain shall be shown on appropriate General Plans and on Plan and Profile Drawings.

2.3.3 Road Details

Horizontal control data (beginning and end of curve, radius, length, etc.) shall be shown on appropriate General Plans and on Plan and Profile drawings.

Vertical control data (proposed road grade, length of run and percent slope, beginning and end of vertical curves, high and low point) shall be shown on Lot Grading Plans and on Plan and Profile drawings. Existing and proposed centreline road elevations shall be shown every 20 metres with stations shown measured in metres with kilometres separated by a + sign on long runs (e.g. - STA 0+000, STA 0+020, STA 0+040...STA 1+020). Stations of interest (curve stations, intersections, end stations, etc.) shall be shown calculated to the nearest millimetre (e.g. -BVC STA 0+041.169, EVC STA 0+066.169, END STA 0+069.124).

2.3.4 Lot Grading Plans

Lot grading plans shall be prepared at a scale of 1:500 and show the information indicated on Township standard drawing ESD-200.01.



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The design for individual lot grades shall be in accordance with Township standard drawings 201.01 and 201.02.

The plans must show overall lot fabric with lot and block numbers, street names, right-of-way limits, road curb and gutters, catchbasins, sidewalks, all aboveground street furniture and driveway locations. All easements and blocks for walkways or maintenance access shall be indicated on the plan complete with dimensions. The plans shall show existing topographic information, including existing contours and contour labels at an interval of 0.50 m and shall extend 30 m beyond the limits of the project.

Road grades shall be shown at 20 m intervals and at all changes in grade. Directional arrows shall be provided indicating the overland flow route.

Existing and proposed retaining walls, including top and bottom elevation, and existing and proposed embankments including slope label and associated elevations shall be shown.

Any lots requiring more than 1.2 metres of fill shall be indicated with an asterisk (*) beside the lot number to show that either engineered fill or extended footings may be required.

Any existing structures shall be shown with notes clearly identifying whether they are to be retained or demolished/decommissioned.

2.3.5 Stormwater Management Pond Facilities

Drawings for SWM pond facilities shall be at a scale which is suitable to clearly illustrate the required details, such as 1:500 (minimum), 1:300, 1:250.

Information to be shown on the SWM pond drawing includes:

- i. Permanent pool volume
- ii. Forebay volume;
- iii. Active storage volume;
- iv. Location and height of berms;
- v. Location, invert elevations and size of pipes, inlets and orifices for the outfall;
- vi. Maintenance access;
- vii. Sediment drying area;
- viii. Contours with storm storage level elevations;
- ix. Cross sections across pond and longitudinally through invert, forebay, pond bottom, outlet control structure and discharge pipe;
- x. Perimeter ground elevations demonstrating compatibility with adjacent properties.

The minimum details to be shown include that for spillway, outlet control structure, control orifice and maintenance access.



2.3.6 Miscellaneous Details

Other details shall be according to the Township Standard Drawings where applicable or if a Township Standard Drawing is not available in accordance with Ontario Provincial Standards. Township Standards take precedence when available. All necessary details shall be included on sheets similar to other drawings sheets, if not on relevant drawings. Township Standard Drawings may be printed on these detail sheets directly.

3 Roadways

3.1 GENERAL

OPSS and OPSD shall apply together with these Township Standards. Where there are any apparent conflicts or discrepancies, the Township Engineering Standards and Standard Drawings shall take precedence.

- i. All roads to be constructed in serviced areas in the Township of Essa shall be designed to urban standards complete with a sanitary sewer system, water distribution system, storm sewer system, curb and gutter, concrete sidewalk and an asphalt surfaced road structure unless specific approval from the Township is received prior to the development receiving draft plan approval.
- ii. Roads shall be classified as local, collector or arterial in accordance with the Township Official Plan.
- iii. Right of ways shall be a minimum of 20 m wide for local streets, 26 m for collector roads and 30 m for arterial roadways.
- iv. Where a development adjoins or incorporates an existing County Road or Township arterial roadway as per the Township's Official Plan, the Developer shall deed to the County or the Township the required property widenings.
- v. The minimum longitudinal grade on any roadway shall be 0.5 percent and the maximum grade shall be as per Section 3.4, Table No. 2.
- vi. Property daylighting corners at intersections of Township roads and County or Provincial Roads shall be in accordance with Section 3.5.3, Table 3.
- vii. The edge of the roadway pavement surface shall have a minimum radius at intersections as per Section 3.4.3, Table No.
 3
- viii. Finished roadways shall have a crossfall of 2 percent conforming to OPSS geometric design standards.
- ix. 150mm diameter perforated road subdrains complete with filter sock are required full length under curb lines as part of the road structure design.
- x. Where a development road terminates adjacent to a future development phase or stage, a temporary turn around will be required conforming to Township standard ESD-103.02.
- xi. The road design for industrial and/or commercial developments shall take into account the type of traffic anticipated on the development. Granular base thicknesses, asphalt type and thickness, shoulder width and cul-de-sac radii shall be designed specifically for the development utilizing these standards as minimum requirements.
- xii. Where new roads are to connect to existing roads, the design shall extend along the existing road for a sufficient length to provide a satisfactory transition.
- xiii. All roads are to be extended to the limit of the subdivision boundary and shall terminate at a cul-de-sac conforming to Township standard ESD 102.01 when not connecting to an existing road.



- xiv. Provisions shall be included in the road design for communal mailboxes. The Developer will be responsible for providing parking areas, structural concrete foundations, electrical supply, etc., all as required by the Township and Canada Post Corporation for communal mailboxes in locations designated by the Township and Canada Post.
- xv. All new residential developments require provisions for off-street visitor parking facilities, the location and size of which will be determined in conjunction with Township staff at the time of development application.

3.2 TRAFFIC CALMING

The Township requires the implementation of traffic calming measures in a development which are deemed appropriate for the neighbourhood, the roadway classification and in accordance with the Township's Transportation Master Plan (TMP) guidelines for suitable traffic calming measure designs and implementation. The TMP should be referred to for approved measures and design requirements.

3.3 PAVEMENT DESIGN

For all development proposals, a geotechnical investigation and report must be prepared by a qualified engineering consultant which recommends pavement designs for each pertinent class of roadway.

For reference, the <u>minimum</u> pavement structure to be met for each classification of Township road is provided in the table below:

Table 1: Minimum Pavement Structure

| CLASS | SURFACE ASPREMENT | BINDER/ASPHAND | BASECONTASE | SUBHSASE COURSE |
|------------|-------------------|----------------|---------------------|---|
| Local | 40 mm HL3 | 50 mm HL4 | 150 mm Granular 'A' | 300 mm Granular 'B' |
| Collector | 50 mm HL3 | 90 mm HL4 | 150 mm Granular 'A' | 450 mm Granular 'B' |
| Arterial | 50 mm HL3 | 90 mm HL4 | 150 mm Granular 'A' | 450 mm Granular 'B' |
| Industrial | 50 mm HL3 | 50 mm HL4 | 150 mm Granular 'A' | 300 mm or thicker depending on Geotechnical Recommendation |
| ् द | | | | |

The use of Superpave may be permitted for any road class; however, a recommendation must be made by the geotechnical consultant and must be approved by the Township.

The pavement structures identified above are a minimum and the actual pavement structure may need to increase as a result of the following:

- i. Local conditions such as soil type or water table;
- ii. Anticipated traffic loading;

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- iii. To ensure that there will be no half load restrictions on collector, arterial, or industrial roadways; and
- iv. Any other requirements identified in the geotechnical report.

All asphalt materials and work shall conform with OPSS 1150.

3.4 **GEOMETRIC ELEMENTS**

The following geometric standards shall be applied:

Table 2: Geometric Standards for Road Design

| ROAD CLASS (CROSS - | ROW DESIGN (M) SPEED (M) (MA(41) | | PAVEMENT MIN./ WIDTH ¹ (M) CDAD(#) | MAX. | MIN. HORIZONTAL C/L | VERTICAL CURVATURE MIN.:'K' VALUE? | | MIN. TANGENT BETWEEN HORIZONTAL |
|------------------------|--|--------|--|----------------|---------------------------|--|----------------------------|---------------------------------------|
| SECTION) | с. (КЛ ³ 1 | (KM/H) | :M/H) | GRAD (%) | CURVATURE (M) | CREST (M) | 。这时,这一次全部一百姓的 ⁴ | CURVES (M) |
| | | | | Urban | | | | |
| Arterial | 30 | 80 | 14.0 | 0.5/5.0 | 340 | 26 | 30 | 120 |
| Collector | 26 | 70 | 14.0 | 0.5/6.0 | 190 | 17 | 23 | 80 |
| .ocal | 20 | 60 | 8.5 | 0.5/6.0 | 20 | 11 | 18 | 50 |
| Industrial | 26 🔦 | 60 | 10.0 | 0.5/6.0 | 40 ³ | 11 | 18 | 50 |
| | | | Se | emi-Urban / Ri | ural | | <u>,</u> | |

0.5/6.04

0.5/6.04

30

40³

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11

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18

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50

8.0 ¹ "Sawtoothing" of a roadway is not preferable and will require Township approval on a case by case basis.

7.5

² K values less than those listed above may be considered at stop approaches.

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³ Pavement widening required on horizontal curves as per TAC Geometric Design Guide for Canadian Roads.

⁴ On rural roads, the cross-section will include curb & gutter where centreline grades are equal to or greater than 5%.

3.5 **OTHER DESIGN ELEMENTS**

3.5.1 Vertical Curves

Local

Industrial

20

26

For local streets, all changes in grade of 2.0% or more shall be accommodated through the use of a vertical curve.

For all other road classifications, a change in grade of 1.0% or more shall be accommodated through the use of a vertical curve.

3.5.2 Crossfall

Finished roadways shall have a minimum crossfall of two percent (2%). Super-elevated road platforms shall be considered subject to site specific conditions.

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3.5.3 Curb Radii and Daylighting Requirements

The curb return radii measured from edge of pavement, and daylighting requirements at intersections shall be in accordance with the table below:

| en e | an in the second star of the second star and the second | And the Construction of the second structure | APISUADE. | an an an ann an tha an |
|--|---|--|----------------------|--|
| ROAD CLASS | INTE | RSECTING ROAD CLAS | 5 MIN. CURB RADII (M |) DAYLIGHTING (M) |
| Local | | Local | 9 | 3x3 |
| | | Collector | 9 | 5x5 |
| | | Arterial ¹ | 9 | 10x10 |
| Collector | | Local | 9 | 5x5 |
| | | Collector | 12 | 10x10 |
| | | Arterial ¹ | 15 | 10x10 |
| Arterial | | Local | 9 | 10x10 |
| | | Collector | 15 | 10x10 |
| and the second sec | | Arterial ¹ | ²(see below) | 15x15 |
| Industrial | | Any Other Class | 18 | 15x15 |

Table 3: Minimum Curb Radii and Daylighting Requirements

¹ If the intersecting arterial road is a County Road, County of Simcoe design requirements will govern.

² A compound curve shall be provided in accordance with M.T.O. Geometric Design Standards for Ontario Highways to accommodate a Wb-15 vehicle

3.5.4 Cul-de-Sacs

Cul-de-sacs are to be provided at all dead ends, where there is no proposed connection to an existing street, in accordance with Township standard ESD-102.01.

3.5.5 Community Mailboxes

Provisions shall be included in the design for community mailboxes. While the final locations are subject to approval by the Township and Canada Post, the designer must propose locations that are conducive to the pedestrian and vehicular movements anticipated in the area. As a result, placement on collector or arterial roads should be avoided. Further,



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mailboxes are to be located a minimum of 30 m from any intersection, preferably on a lot line and giving consideration to other aboveground features such as streetlights, fire hydrants, transformers, etc. Community mailboxes must have direct sidewalk access wherever possible and are to face the sidewalk (if present).

3.5.6 Transitions

Where a new street is to connect to an existing street, whether the design includes an extension of the existing street or connects at an intersection, the design shall extend along all existing streets for a sufficient length to provide a satisfactory transition. In the case of an intersection, the distance must be equal to the greater of the required stopping sight distance or the turning lane development lengths, if applicable, for all legs of the intersection.

3.5.7 Road Base Construction and Sub-Drains

The sub-grade shall be excavated or filled to the required grade for the required width of road surface plus shoulders or curbs plus 0.6 metre or the additional width necessary for the required depth of granular road base. Where earth fill is required, it shall be placed in lifts not exceeding 150mm in depth and each lift shall be thoroughly consolidated to the required density and approved by a qualified geotechnical consultant.

All vegetation, boulders over 150mm in diameter, topsoil and organic or frost-heave susceptible materials shall be removed from the road-base to a depth of 1.0m below finished grade and replaced with suitable granular fill material. In swamp or other areas where the depth of unsuitable material exceeds 1.0m below finished grade, the soils report shall make specific recommendations for consideration by the Township Engineer.

All unsuitable excavated material shall be removed entirely clear of the road allowance. The deposition of any fill type material within the Township is to be in compliance with the Township's cut and fill by-law requirements.

The sub-grade shall be shaped to conform to the required longitudinal grade and cross-section and shall have a cross-fall of 3 percent from the centreline of roadway to each side. If considered necessary by the Geotechnical Consultant, the sub-grade shall be compacted with suitable mechanical compaction equipment as required to produce a solid base for the road gravel. All structurally weak sub-grade soils shall be excavated and backfilled with granular base material or in accordance with the recommendations of a geotechnical consultant and approved by the Township's engineer. The granular base shall be laid on dry, smooth, properly graded sub-grade and shall be spread for the required width of surface plus shoulders and tapered at the edges to meet the edge of sub-grade.

The granular materials shall be spread in layers of 150mm maximum compacted depths, or at lesser depths if recommended by the Soil Report, and each layer shall be thoroughly compacted to specifications.

Recycled concrete will not be permitted as granular road base.

No granular base or surface material shall be placed until the grade on which it is to be laid has been subjected to a proof role and approved by the geotechnical consultant and Township Engineer.

During and between construction seasons, the granular base shall be maintained suitable for vehicular and pedestrian traffic including dust control to the satisfaction of the Township Engineer.



Following the placement of base course asphalt, the Developer shall be responsible for weekly power washing and sweeping the road surface until such time as the boulevards are stabilized to the satisfaction of the Township.

Continuous sub-drains, full length beneath curbs, shall be provided for all road designs on both sides of the road base and shall consist of 150mm diameter (minimum) perforated, corrugated polyethylene piping with a geotextile filter fabric. Sub-drains shall be bedded in a 300mm x300mm trench below and at each edge of the sub-grade and shall conform to O.P.S.S. Sub-drains are to be connected to catchbasins. When a sub-drain is extended to drain above grade, it shall be provided with a 3m section corrugated pipe end section and rodent grate.

3.5.8 Curb and Gutter

For all new developments concrete curb and gutter is to conform to OPSD 600,070 (two-stage).

Concrete curb and gutter shall be in accordance with OPSS 353, with the exception that all concrete shall be supplied in accordance with the "Performance Specification Alternative" of OPSS.MUNI 1350 and shall meet the requirements of CSA A23.1, Exposure Class C-2, with a minimum compressive strength of 32 MPa at 28 days.

Prior to the placement of the top stage curb, the base curb is to be completely cleaned and reviewed by the Township Engineer. Any found defects are to be corrected to the satisfaction of the Township. The developer shall make every effort to protect the curb and gutter during the maintenance period. Prior to the placement of top asphalt, the curb and gutter is again to be reviewed by the Township Engineer. Any curb and gutter found to be cracked, chipped, or deficient in any way is to be removed and replaced all at the developer's expense.

Curb and sidewalk shall be extended across commercial and industrial entrances and according to OPSD 350.010 unless the entrance is signalized or is aligned with a municipal street on the opposite side of the street.

Treatment at residential entrances shall be according to OPSD 351:010.

3.5.9 Grading within Right-of-Way

The boulevard area between the curb and the street property line shall be graded to provide positive drainage toward the roadway at a minimum 2% grade.

For roads having a rural design section, the area between the edge of the road shoulder and the street property line shall be graded and the ditches cut with slopes of 3m horizontal to 1m vertical from the edge of the shoulder to the bottom of the ditch and from the bottom of the ditch to the original ground. In fills over 1.5m measured vertically from the edge of shoulder to the toe of slope, the fill slope shall not be steeper than 2:1. The ditch shall be located at the toe of the fill slope. On fills higher that 3.0 metres measured vertically from the edge of boulevard to the toe of slope, steel beam guide rails shall be installed conforming to the Ontario Provincial Standard Drawings and Ministry of Transportation protection warrants.

All side slopes, ditches and boulevards to the street line shall be provided with a minimum 100mm of topsoil and nursery sod. Hydraulic seeding of ditches and boulevards will be accepted for the issuance of the Certificate of Substantial Completion within estate residential developments. Prior to the issuance of the Certificate of Final Completion, the ditches and boulevards must have an established growth of sod subject to the satisfaction of the Township.



3.5.10 Road Crossing and Entrance Culverts

The minimum sizes for driveway entrance culverts and road crossing culverts are 400 mm.

The capacity of the culvert must be sufficient to ensure a conveyance of 1:5 year for driveway culverts and 1:25 year for road crossing culverts. Where the conveyance capacity of the minimum size culvert is not sufficient, a larger diameter culvert is to be sized using the cross-sectional end area calculated by the rational design method. Road crossing culverts are to be sized in accordance with MTO drainage Design Standards, WC-1 Design (Bridges and Culverts). Detail drawings and calculations shall be submitted for approval by the Township.

4 Builders Road

A "builder's road" must be constructed prior to issuance of a building permit. A road will be classified as a "builder's road" when the granular bases and sub-drains, curb and gutter, base course asphalt and rough grading of the boulevards, including street name signs and traffic signs, has been certified complete by the Developer's Engineer and acceptable to the Township Engineer.

5 Driveway and Entrance Design

5.1 DRIVEWAY CULVERTS

The minimum length of culvert shall be that which is required to extend under the driveway with matching slopes from top of culvert to driveway edge not exceeding 3:1.

The minimum depth of cover for all culverts is 300 mm unless otherwise indicated by the Township.

Culverts shall be constructed with granular bedding and backfill in accordance with OPSD 802.014 or 802.034.

Culverts 600 mm and smaller in diameter shall be HDPE. Larger than 600 mm diameter are to be CSP.

5.2 GRADES

The maximum grade for driveways and entrances shall be 7% except in site specific cases that receive written approval from the Township. This maximum grade is not recommended and should be employed only in exceptional cases where conditions prohibit the use of lesser grades. The minimum grade permitted is 2%. Maximum grade change shall be 4% at curb or sidewalk and 1% per 2.0 metres average.

5.3 LOCATION

5.3.1 Sight Distance Requirements

New accesses on Township roads must meet the following minimum sight distance requirements, which are consistent with those provided in the TAC Geometric Design Guide for Canadian Roads.

| DESI | GN MINIMUM STOPPING SIGHT — | INTERSECTION SIGHT DISTANCE (M) | | | |
|--------------|-----------------------------|--|---------------------|--|--|
| SPEE (KM/ | DISTANCE (NA) | Right Turn from Stop/ Crossing Maneuver | Left Turn from Stop | | |
| 50 | 65 | 95 | 105 | | |
| 60 | 85 | 110 | 130 | | |
| 70 | 105 | 130 | 150 | | |
| 80 | 130 | 145 | 170 | | |

Table 4: Sight Distance Requirements

5.3.2 Clearances

All access driveways shall be located with a minimum clearance of 1.5 metres to utility poles, hydro transformers, hydrants, telecom junction boxes, side lot lines and other driveways. Water service valve boxes are not permitted in driveways and shall be installed a minimum of 1.0m from the edge of the driveway. On roadway bends and in cul-de-sacs, the extensions of the property line to curb line are to be at least 1.0m offset from edge of driveway. Where frontage limitations interfere with standard locations, site specific solutions shall be detailed with the Plan and Profile and Lot Grading Plans. On corner lots, the edge of driveway is to be a minimum of 4.5 metres from the daylight triangle corner.

5.4 WIDTH

The width of a typical residential driveway shall be the lesser of half the width of the lot to a maximum driveway width of 6 metres. For lots less than 9m the maximum driveway width shall be 3.5m.

A secondary driveway entrance will only be permitted for estate residential lots with frontages exceeding 30 metres with the provision that the total driveway width of both driveways does not exceed 30% of the total lot frontage.

Deviations from the standard driveway widths may be considered subject to the following site considerations:

- i. width of road and grade;
- ii. end slope on entrance;
- iii. depth of ditch;
- iv. turning radius;
- v. sight lines visibility;
- vi. proximity to existing entrances;
- vii. total road frontage;
- viii. volume and types of traffic;
- ix. divided access lanes;
- x. number of lanes of traffic.

5.5 RADIUS

For non-residential entrances, the minimum driveway curb radius is 9.0 m but may be greater to accommodate truck traffic depending on the intended site use. The end of the curb radius at street line should not extend beyond property line.

5.6 GRANULAR BASE AND ASPHALT

For residential driveways, between the back of curb and property line, the driveway entrance shall have a minimum of 150mm of Granular 'A' and 50mm HL3 surface asphalt, or other hard surface as approved by the Township.

For non-residential entrances, the minimum pavement structure shall be 300mm of Granular 'B', 150mm of Granular 'A', 50 mm HL4, and 40mm HL3. This minimum pavement structure shall also apply for the entire fire route.

The foregoing are minimum requirements and a more stringent pavement structure may be required and must be confirmed by the Geotechnical Engineer.

6 Sidewalks

A 1.8m metre wide concrete sidewalk shall be placed in locations approved by the Township. In general sidewalk will be required on one side only for local roads and on both sides for collector and arterial roads. Except for collector road where sidewalks are set 1.0 m from property line, the standard location is to set the edge of sidewalk 0.5 m from property line. The closest edge of sidewalk location to edge of pavement in Township standards is 3.45 m.

All sidewalks shall be constructed on a properly constructed foundation of a minimum of 150mm of compacted Granular "A". The sidewalk shall be 150mm thick except at commercial and industrial entrances where it is to be 180mm thick. All concrete materials and work shall conform to OPSS 351. Concrete shall be 32 MPa, Class C2 in accordance with CSA A23.1-04.

Tactile walking surface plates shall be inserted in the sidewalk at street intersections in accordance the requirements of the Accessibility for Ontarians with Disabilities Act (AODA). Tactile walking surface plates are to be subject to Township approval. Concrete sidewalk and sidewalk joints are to be installed as per OPSD 310.010.

Prior to the issuance of the Certificate of Final Completion and Final Acceptance the sidewalk is to be reviewed by the Township Engineer. Any sidewalk found to be cracked, chipped, heaved, defaced or deficient in any way, is to be removed and replaced all at the developer's expense.



7.1 WALKWAYS

The following identifies general requirements for typical walkways. It is recommended that discussion with the Township be carried out in the planning stages to determine walkway requirements. All walkways are to be provided with 1.5 meter high chain link fence unless they are adjacent to a school or storm water management facility. All pedestrian walkways are to be provided with signs indicating, "No Winter Maintenance" and "No Motorized Vehicles"

7.1.1 Pedestrian Walkway (No Landscaping)

Where the walkway block serves only pedestrians, the walkway block is to be 3 metres wide with chain link fence on either side. The walkway shall be asphalt or concrete at the Township's discretion depending on the location and walkway purpose. In general, the sidewalk standards will be applied to the asphalt or concrete structure. Pedestrian entrance gates may require a removable bollard at the Township's discretion.

7.1.2 Pedestrian/Lawn Maintenance Pathway (No Landscaping)

If the pathway is to be used as an entrance to an Open Space Block or Park Area and it will be used by lawn maintenance equipment, then a 4 metre wide block shall be provided. An asphalt walkway 4 metres in width will be provided with chain link fence along each side. At the Township's discretion, a pedestrian entrance gate with removable bollard may be required at street line. The distance between the gate posts will be 4 metres.

7.1.3 Walkway/Utility Corridor

Where the walkway block includes trunk services for watermain, sanitary or storm sewer services, the block shall be a minimum of 6 metres wide. It will include a 3 metre wide asphalt walkway and a 1.5 metre wide landscaped strip along both sides. At the Township's discretion, a pedestrian entrance gate with removable bollard may be required at street line. The gate posts will be 4 metres apart. Additional wooden bollards may be required beyond the limits of the walkway to prevent motorized vehicles from accessing the walkway.

The spacing, depth and size of the trunk services shall also be considered and, if necessary, the walkway block is to be widened. The horizontal separation of services is to meet the recommendations of the MECP and the centreline of the service must not be closer than 1.5 metres from the limit of the walkway block.

7.2 FENCING

The requirements for fencing shall be determined during the drawing review process and consider safety and privacy requirements. In general, a 1.5m high standard galvanized chain link fence will be constructed with a top and bottom rail and 38mm wire mesh consisting of 9 gauge (3.4mm) steel. Galvanized chain link fence shall be placed at locations along all walkways, open spaces, parks, and steep slopes, adjacent to commercial properties and as required by the Township. In some instances, a 1.8-metre-high wood privacy fence may be required between the new and existing development or between



residential and commercial properties or between commercial and institutional properties. When abutting Township property, privacy fence shall be installed on the private property side of the property line.

Where required, the location and extent of acoustical fence shall be defined in the noise and vibration study prepared for the development. Various types of acoustical fence will be considered during the review process. Acoustical fence shall be installed on the private property side of property line.

Where an Open Space Block is adjacent to a municipal road allowance, a fence will not be required unless necessary as part of a Park Plan or to provide a barrier from steep grades.

Fencing of storm water management facilities will be at the Township's discretion. In general, storm water management facilities are to be designed with maximum slopes of 5:1 in order to avoid fencing requirements.

8 Utilities, Streetlighting, and Traffic Signals

8.1 GENERAL

The location of all utilities including communications, cable, gas, streetlight and hydro must be installed in a common utility trench, in accordance with the locations as detailed on the typical cross section.

Anyone intending to occupy or perform work on Township of Essa property must first apply to the Public Works Department for a Road Occupancy Permit. Approval must be obtained from the Manager, or his/her designate, prior to the commencement of work. Full restoration requirements for road cuts and boulevard repairs following utility installations in Township property shall be in accordance with Conditions of Approval in the Township's Road Occupancy Permit Application Form.

8.2 STREETLIGHTING

All streetlight lamps to be light emitting diode (LED).

Streetlights shall be installed throughout the subdivision and may also be required on roadways bordering the development if deemed necessary by the Township. A qualified engineering consultant shall prepare the streetlight design and provide a photometric plan for the proposed layout. All streetlight design must conform to the requirements of the Illumination Engineering Society of North America Design Guidelines (or any successor body).

The installation of the street lighting system shall be in compliance with ESA, CSA, Hydro One requirements, the Township's standards and specifications and manufacturer's specifications.

Poles must be installed on projections of lot lines. In urban subdivisions, all wiring is to be in underground ducts.

The location of the streetlights should generally be on the south and east side of the road, opposite the watermain. The poles shall be installed to the outside of road curves.

Streetlight poles, luminaires and arms shall be in accordance with the Approved Products List in Appendix B. Product substitutions must receive prior approval from the Township.

Streetlight types will be either Standard Street Lighting (Cobra Head on arm) for urban residential developments or Decorative Street Lighting (Side mount coach style luminaire) for estate residential developments.

General streetlight product requirements and principles are as follows:

- 8.2.1 Standard streetlighting pole and arm poles to be direct buried Class "B" pre-stressed round concrete with provisions for electrical ground. The streetlight arm shall be tapered elliptical aluminum bracket.
- 8.2.2 Decorative streetlighting pole and arm poles to be direct buried octagonal spun concrete with polished black finish and anti-graffiti coating or approved equivalent. The streetlight arm shall be aluminum, black polished finish.
- 8.2.3 Luminaires only full cut off and International Dark Sky Association (IDA) compliant fixtures shall be accepted.

The individual LEDs shall be connected such that a loss or a failure of one LED will not result in the loss of the entire luminaire.

Each luminaire shall have the manufacturer's name, trademark, model number, serial number, date of manufacture (monthyear), rated voltage in watts, and lot number as identification permanently marked on the back of the module. Each streetlight shall be provided with an identification number as provided by the Township.

A photoelectric unit shall be supplied for each luminaire. The design of the photoelectric unit must include features to ensure that the device remains insect free.

Light fixtures should be positioned across a site so as to give a uniform distribution of light across the relevant area to assist in the avoidance of the creation of "hot spots," being areas of over-illumination that make adjacent areas seem darker.

Fixtures shall be positioned such that they focus light down, preventing light from emitting above the horizontal plane (90degree position relative to the ground) and preventing light trespass.

Lamps shall be located in such a way to direct light away from neighbouring properties.

Streetlight power distribution pedestals - heavy gauge galvanized steel with powder coated paint finish and concealed ventilation.

Ducts - shall be solvent welded together in trenches with minimum cover of 600mm. Ducts shall be surrounded by 80mm of brick sand and warning tape to cover width of trench.

Road crossings of existing roads are to be carried out by directional bore method with a minimum depth of cover of 750mm using thickwall PVC duct.

Where open-cut road crossings are necessary, ducts shall be installed as per OPSD 2100.06 and OPSS 603.

8.3 TRAFFIC SIGNALS

8.3.1 General

Traffic signal design is to be prepared by a qualified engineering consultant in accordance with the Ontario Traffic Manual Book 12 and the appropriate electrical design codes. All work shall be performed in accordance with the current edition of the Ontario Electrical Code, including all appending bulletins issued by the Electrical Safety Authority which are applicable to the work. All work shall be governed by Federal, Provincial and Local laws and by-laws pertaining to the work, as well as by the latest issue of CSA Standards pertinent to the work. All electrical work is subject to inspection by the Electrical Safety Authority. In the event of a conflict between regulations, the strictest regulation shall apply.

In addition to accommodation of traffic flow, the signalized intersection design shall include pedestrian requirements and accessibility features. All signalized crossings must be designed in accordance with the TAC "Guidelines for the Understanding, Use, and Implementation of Accessible Pedestrian Signals". Accessibility features must include gentle grades for sidewalks, drop curbs and directional lines in the sidewalk to provide guidance for the visually impaired. Pedestrian crosswalk features shall include single head "walk/don't walk" heads, countdown heads, and audible pedestrian heads with tactile push button.

9 Signs and Pavement Markings

9.1 STREET NAME SIGNS

The Developer will be responsible for coordinating the supply of the street name signs with the Township. All costs associated with the supply and installation of the signs will be borne by the developer. Street name signs of an approved design in green colour shall be erected on approved metal posts (U-Channel galvanized), complete with break-away flange, 3.6m long and embedded 1.2m in the ground. Street name signs are not to be mounted on stop sign posts and are to be installed on the opposite side of the road as the stop signs.

9.2 TRAFFIC SIGNS

Signs of the standard type conforming to the Ontario Traffic Manual Book 2 shall be mounted on 3.6 m (1.2 m lower channel and 2.4 m upper channel), double-slide, and "U-Channel" galvanized steel posts, embedded 0.9 m in the ground. The signs shall be located as required by the Township after a By-law is passed for their installation.

9.3 PAVEMENT MARKINGS

Upon completion of the final asphalt and upon notification by the Township, pavement markings shall be painted conforming with the Standards of the Ontario Traffic Manual, Book 11 (Pavement, Hazard and Delineation Markings) at all intersections, school crossings, walkways and railway crossings to clearly indicate the proper traffic zones, lanes and stop lines.

Lane markings are to be organic solvent based or water borne traffic paint complete with glass beads. All lane marking applications to new asphalt require two applications of paint. The second application shall not be applied until the first is tack free. Pavement markings are to conform with OPSS 532 and OPSS 1712.

Traffic lane symbols, stop bars and pedestrian crosswalks are to be durable pavement markings or field reacted polymeric pavement markings in accordance with OPSS 532, OPSS 1713 AND OPSS 1714.

All pavement marking removal required to prepare the area for final pavement marking shall be done by abrasion.

Local streets do not require centreline pavement marking, but stop bars are required with a minimum of 15 metres centreline marking.

9.4 SEASONAL ACTIVE TRANSPORTATION LANES

The requirements for Seasonal Active Transportation Lanes are to be discussed at the planning stage and within the Traffic Impact Study.

For collector or arterial streets, the Township may require the provision of a Seasonal Active Transportation lane in accordance with the Township's Transportation Master Plan.

Dedicated Seasonal Active Transportation lanes are generally not required on local streets.

The design for the Seasonal Active Transportation lanes shall be in accordance with the Transportation Association of Canada (TAC) Urban Supplement to the Geometric Design Guide for Canadian Roads and the Bikeway Traffic Control Guidelines of Canada.

The designer shall confirm with the Township if there will be any requirements for the provision of Seasonal Active Transportation Lanes prior to the commencement of the engineering design.

10 Storm Drainage and Stormwater Management

10.1 GENERAL

This section outlines the policies, design criteria and applicable parameters for the design and implementation of storm drainage facilities within the Township. These policies are to be adhered to in the design of all stormwater management facilities, including sewers, surface drainage and related environmental facilities.

In particular, the principles and design criteria in this section are intended to be consistent with and augment the guidelines, policies, and standards established in the latest version of each of the following:

- Ministry of Transportation Drainage Management Manual (1997),
- Ministry of the Environment Stormwater Management Practices, Planning and Design Manual (2003),
- Nottawasaga Valley Conservation Authority Development Review Guidelines (2006),
- Ministry of the Environment Phosphorous Reduction Strategy (2010).

Accordingly, the following objectives are to be observed and achieved for drainage management within Township boundaries:

- i. Prevent loss of life and minimize property damage and health hazards;
- ii. Minimize inconvenience from surface ponding and flooding
- iii. Minimize adverse impacts on the local groundwater systems and base flows in receiving watercourses;
- iv. Mitigate downstream flooding and erosion;
- v. Minimize pollution discharges to watercourses;
- vi. Minimize soil losses and sediments to sewer systems and water bodies from construction activity;
- vii. Minimize impairment of aquatic life and habitat;
- viii. Promote orderly development in a cost-effective manner.

In the planning and design of stormwater management facilities, the designer is to have full regard for the riparian rights of all surrounding, upstream and downstream landowners. Consideration of the surrounding topography, land uses, environment and integration of the facility into all such elements is essential.

10.2 MINOR SYSTEM

Generally, storm drainage shall be accommodated by a system of curb and gutters as well as storm sewers, with the exception of rural roadways or estate residential subdivisions where an open ditch system may be permitted according to conditions and subject to prior approval from the Township.

Storm sewers are to be designed to convey a minimum of the 1:5 year storm without surcharge. However, when the major storm drainage system is inadequate the Township may require a storm sewer system designed to carry additional flows.

10.2.1 Service Area

The storm drainage system shall be designed to accommodate all on-site drainage areas as well as all external tributary areas to their maximum future development capacity in accordance with the Township's Official Plan, as determined by the delineation of appropriate topographic mapping and preparation of drainage plans.

10.2.2 Design Flow

Storm sewer systems with a drainage area < 40 ha shall be designed to convey the 1:5 year (minimum) design storm using the Rational Method and the Township's IDF regression equation for rainfall intensity unless otherwise approved or directed by the Township. Storm sewer systems with a drainage area > 40 ha shall be designed using an approved computer program and verified with the Rational Method. The storm sewer design shall be based on the larger of the two flows calculated using the computer model and the Rational Method. Under no circumstances shall the storm system be designed in a surcharged condition.

All storm sewers shall be designed according to the Rational Formula where:

Q = (C)(i)(A)

Where: Q = design flow (m³/s) C = the site specific runoff coefficient I = average rainfall intensity (mm/hr) A = drainage area (ha)

Average Rainfall Intensity, I

The average rainfall intensity shall be calculated in accordance with the following table and equation:

Table 5: Rainfall Intensity Return Periods

| PARAM | AETER 2-YR | .5-YR | 10-YR | 25-YR | 50-YR | 100-YR |
|-------|------------|---------|---------|----------|----------|----------|
| А | 678.085 | 853.608 | 975.865 | 1146.275 | 1236.152 | 1426.408 |
| В | 4,699 | 4.699 | 4.699 | 4.922 | 4.699 | 5.273 |
| С | 0.781 | 0.766 | 0.760 | 0.757 | 0.751 | 0.759 |

Parameters based on rain gauge data for the period 1979 – 2003 for the Barrie WPCC Station #6110557.

The IDF intensity values for Barrie WPCC Station were increased by 15% before calculating a, b, c values to account for climate change.

Rainfall Intensity Calculation

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l = A/(t+B)^C

Where: I = intensity (mm/hr)

t = storm duration (minutes)

A, B, C = a function of the local intensity-duration data

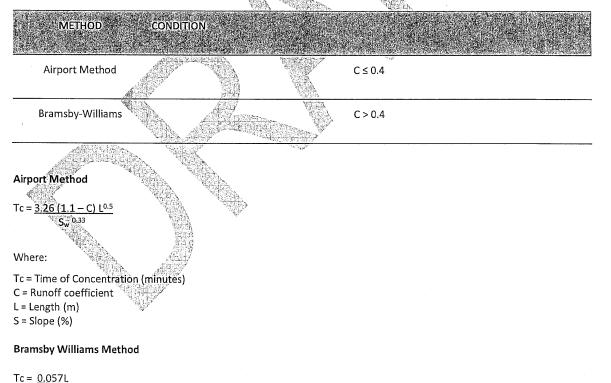
The storm duration is set to the time of concentration (i.e. the sewer inlet time plus the time of travel in the pipe or channel) for the total cumulative drainage area to the node of interest. The maximum inlet time for the first pipe of a storm sewer system is 10 minutes.

This shall apply where the upstream drainage area does not include large open space areas. Where peak flows from external areas enter a subdivision sewer system, the more critical case based on either the time of concentration including the external area or the time of concentration excluding the external area shall be used. Actual velocities of computed peak flows shall be used to estimate time of concentration.

Time of Concentration, Tc

The initial time of concentration, to be used to determine total time of concentration,

should be calculated using the following methods, according to the run-off coefficient as indicated:



 $S_w^{0.2}A^{0.1}$

Where:

Tc = Time of Concentration (minutes) L = Length (m) S = Slope (%) A = Area (ha)

Typical initial Tc value for urban areas is 10 minutes and should be used as the minimum value.

Runoff Coefficient, C

An arithmetic weighted C is used where there are different land uses and soil types to obtain a composite runoff coefficient. as shown below:

| С | = | <u>(A1C1) + (A2C2) +</u> |
|---|---|--------------------------|
| | | At |

| Where: | C | - | composite runoff coefficient (dimensionless) |
|--------|------|---|--|
| where: | L | = | |
| | A1,2 | = | area corresponding to specific land use or soils type (ha) |
| | C1,2 | = | runoff coefficient corresponding to A1,2 (dimensionless) |
| | At | = | total drainage area (ha) |
| | | | |

Source: adapted from the Ministry of Transportation Drainage Management Manual, 1997, Equation 8.10.

The minimum site-specific runoff coefficients for 5-yr to 10-yr events to be used for the proposed development land uses are per the following table:

| LAND USE | C |
|---|------|
| Residential, single family detached | 0.50 |
| | |
| Residential semi-detached | 0.60 |
| | |
| Residential townhouse | 0.70 |
| | |
| Commercial | 0.90 |
| | |
| Industrial | 0.75 |
| | |
| Grassed park, unimproved (buffer) areas | 0.25 |
| | |

For estimating flows from storms larger than the 5-year event, the runoff coefficients should be increased to account for the increase in runoff due to saturation of the soil, with the estimate becoming less accurate for larger storms. Coefficients for the larger storms can be derived as follows:

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| Return Period | Runoff Coefficient "C" |
|---------------|------------------------|
| 25 years | C25 = 1.1*C5 |
| 50 years | C50 = 1.2*C5 |
| 100 years | C100 = 1.25*C5 |

10.2.3 Pipe Capacity

Manning's Formula is to be used in calculating the full flow capacity of the storm sewer.

The roughness coefficients to be used in the calculation are as follows:

- Concrete pipe, n = 0.013
- PVC and Smooth walled PE pipe, n = 0.013
- Corrugated pipe (for culvert use only), n = 0.024
- 10.2.4 Velocities

The minimum velocity in the pipe under full flow conditions is 0.6 m/s for the 5-year return storm.

The maximum velocity in the pipe under full flow conditions is 4.5 m/s for the 5-year return storm.

10.2.5 Minimum and Maximum Pipe Slopes

The minimum pipe slope shall be that which is required to meet the minimum velocity, but shall not be less than 0.25%. The maximum pipe slope shall be that which is required to meet the maximum velocity.

10.2.6 Minimum Pipe Sizes
The minimum pipe size is 300 mm

10.2.7 Minimum Depth of Cover

The minimum depth of cover to be provided shall be 1.2 m to the obvert of the sewer provided there are no conflicts with utility crossings. Insulation to address shallower installations will be considered only on a localized basis when necessary to address a constraint and is subject to prior Township approval.

10.2.8 Bedding and Backfill

All storm sewers are to be installed with bedding and pipe cover (well graded OPSS Granular 'A' or as recommended and approved by the Geotechnical Engineer and the Township) and backfill in accordance with OPSD 802.010 or 802.030 to 802.032 as applicable. Compaction is to be a minimum of 95% SPD or as indicated in the approved Geotechnical Report.

In soft or wet conditions, additional Geotechnical Investigation may be necessary to determine the appropriate bedding and backfill measures.

10.2.9 Pipe Clearances

Minimum clearances shall be provided in accordance with MECP guidelines.

10.2.10 Sewer Layout

Storm sewers shall generally be located as per the typical road cross-sections.

Where storm sewers mains are located in easements, the easement width shall be a minimum of 6.0m.

10.2.11 Maintenance Holes

Maintenance holes shall be as per OPSD 701 series and shall be placed at the beginning and end of each sewer line, at changes in pipe size and/or material, and at all designed changes in grade and/or alignment. Radius pipe for large diameter sewers may be permitted where it is beneficial to certain circumstances, subject to prior written approval from the Township.

Blind main line sewer connections are not permitted.

All maintenance holes up to and including 1200 mm diameter are to be pre-benched to spring-line by the supplier. All maintenance holes larger than 1200 mm diameter are to be benched to spring-line on-site after installation.

Frost straps are required in accordance with OPSD 701,100,

The maximum spacing between maintenance holes shall generally be according to the following:

- 300 to 900 mm diameter <u>110 metres</u>
- Greater than 900 mm diameter 150 metres

A sufficient drop shall be provided across each maintenance hole to offset any hydraulic losses. The minimum drops across a maintenance hole shall be as follows:

| CHANGE | IN DIRECTION | | | MINIMUM DROP |
|------------|--------------|---|---|--------------|
| Straight i | run (0°) | | N. S. | 0.03 m |
| 1 - 45° | | | | 0.05 m |
| > 45° | | Â | | 0.08 m |
| | ALC: NOT | | | |

The maximum change in direction permitted in a maintenance hole is 90°.

Drop structures shall be provided when the difference in the inlet and outlet inverts is equal to or greater than 0.9 m that cannot be eliminated by changing sewer grades. Internal drop structures are not permitted.

Safety platforms shall be installed in accordance with OPSD 404.020 for all maintenance hole depths of 5.0 m or greater.



Obverts of inlet pipes shall not be lower than obverts of outlet pipes. Where maintenance holes are located in areas to be flooded by the major storm design and surcharged sewer design is not used, maintenance hole covers shall be of the sealed variety. Where maintenance holes are located where the surcharged sewer design hydraulic grade line is higher than the rim elevation, maintenance hole covers shall be of the bolted variety. In all other areas, standard maintenance hole covers shall be used.

Except for special cases as pre-approved by the Township, the downstream pipe diameter shall always be greater than or equal to the upstream pipe diameter.

10.2.12 Catchbasins

Catchbasins shall be located upstream of pedestrian crossings and not within **1.0** m of any curb depressions or in driveways. Preferably, catchbasins will be installed on projections of lot lines. Double catchbasins shall be located where flows are being received from more than one direction, such as at low points.

The maximum allowable spacing shall be in accordance with the following:

| PAVEMENT WIDTH (I | (Iv | | MAXIMUM SP | ACING (M) |
|-------------------|------------|-------------|------------|--------------|
| | | Slöp | 5≦4.5% | Slope > 4.5% |
| 8.5 | | | 90 | 60 |
| 14.0 | | | 60 | 40 |
| | Pollow was | distant for | | |

Catchbasin capacities shall be determined in conjunction with the overall stormwater management system. On roadways, catchbasins shall have a minimum capacity to pass the runoff from the 5-year return frequency storm. Where the pipe system is required to convey flows in excess of the 5-year return frequency storm, sufficient catchbasin capacity shall be provided to permit the design flows to enter the sewer system. Inlet control devices may be used where the hydraulic grade line needs to be strictly controlled to prevent surcharging of the sewer line and to allow storm sewer house connections.

Where catchbasins are used as inlet controls, spacing shall be determined by design and must be approved by the Township. Leads shall be minimum 250 mm.at 0,7% grade for single catchbasins and 300 mm at 0.7% grade for double catchbasins.

Leads shall connect to maintenance holes where possible and shall have a minimum depth of cover of 1.2 m to the obvert.

10.2.13 Foundation Drainage System

In order to minimize the flow rate from foundation drains, a minimum distance of 0.5 m shall be provided between the underside of the basement floor slab and the measured seasonal high groundwater table. Subdivision applications shall clearly include this difference of elevation at the time of the "Draft Plan Application" and shall be updated during the detailed design process.

A foundation drainage system is to be provided for each residential lot. The designer has three options to choose from:

- A sump pump discharging to an underground storm sewer service connection as per Essa Standard Drawing ESD-202.01
 Sump pump discharge is not permitted over sidewalks.
- ii. A gravity connection to the storm sewer, provided the 100-year hydraulic grade line is minimum of 0.5 m below the finished basement floor elevation of the dwelling.
- iii. A sump pump, or gravity connection to a third pipe foundation drain collection (FDC) system, that is separate from the storm sewer system and which discharges to a sufficient approved outlet. The need and suitability of a FDC system shall be addressed in the geotechnical report for the development.

10.2.14 Storm Service Connections

Single connections for residential lots shall be constructed in accordance with Essa Standard Drawing ESD-1000.02 for each type of residential lot. Connections for commercial, institutional or multiple uses will be considered on an individual basis. The minimum depth of cover is 1.5 m and the minimum slope is 2.0%. Utility duct trench depths must be considered in setting the depth of service connections.

Where a sump pump discharges to a storm service connection, a 25mm air gap shall be provided at the building such that there is no direct connection between the building sump pump outlet and the storm service.

Where it can be shown to the satisfaction of the Township that groundwater conditions at footing level are unlikely to require more than 2 sump basins and pumps for an entire Townhouse block, storm service connections to the internal units in a Townhouse block may be eliminated. However, storm service connections must be installed to the end units.

Single residential storm connections shall be 100mm diameter with a 100mm x 100mm test fitting plugged and braced at the property line.

For new developments, the service shall be extended 2.0 m beyond property line, complete with a temporary, plug, brace, and marker.

Roof leaders shall <u>not</u> be connected to the storm sewer system. Roof leaders are to be discharged to the ground surface onto splash blocks, and flows shall be directed away from the building and into the surface drainage / swale system in such a way as to prevent ponding or seepage into the foundation weeping system. Notwithstanding, the use of inground infiltration pits may be considered subject to supporting geotechnical and engineering considerations.

Where flat roofs are used as in commercial or industrial sites, detention roof hoppers requiring smaller or fewer roof leaders may be used as part of the stormwater management design. No connections are to be installed directly to storm sewers.

10.2.15 Testing

The following testing requirements apply to all new storm sewers. Any sections of sewer or service connections which fail to meet the requirements shall be repaired or replaced at the direction of the Township.

10.2.16 Deflection Testing

All newly installed PVC and Polyethylene storm sewers shall be subjected to deflection testing in accordance with OPSS 410.

Deflection testing is not required for concrete storm sewers.

10.2.17 CCTV Inspection

A CCTV Inspection of the storm sewer shall be completed prior to substantial completion and final assumption. CCTV inspection is to be completed in accordance with OPSS 409, including flushing and cleaning.

10.2.18 Visual Inspection

All maintenance holes shall be visually inspected by the Township for deficiencies prior to substantial completion and prior to final assumption.

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10.3 MAJOR SYSTEM

Runoff rates in excess of the design capacity of the minor system shall be conveyed via roadways, swales, walkways, drainage easements, and in special circumstances the storm sewer system, to a sufficient outlet. The combination of the overland flow system and the minor system shall be designed for a minimum 25-year return frequency storm, to prevent flooding of private property with maximum level of road flooding and surface detention as defined in the table below.

It shall also be demonstrated that overland flow conditions resulting from the 100-year return frequency storm will not cause unacceptable flooding damage to private property with a maximum level of road-flooding and surface detention as outlined in the table below. A continuous overland flow route is to be identified on the Engineering Drainage Plans.

In new subdivisions the limit of overland flow route floodlines shall not extend onto private property unless protected by a drainage easement. Development of the site must not increase flood levels upstream or downstream of the development.

For all classes of road, the product of depth of flow at the gutter (m) times the flow velocity (m/sec) shall not exceed 0.65m2/sec.

Street grading must provide a continuous gradient to direct street flows to a safe outlet at low points. Outlets can be walkways or open sections of roadways leading to parks, open spaces or river valleys.

Table 6: Maximum Allowable Flow Depths

STORM RETURN FREQUENCY (YEARS)

| Walkways | minor surface | as required | as required |
|-------------|--------------------------------|---------------------------|---------------------------|
| Open Spaces | flow up to 25mm on walkways | for overland flow outlets | for overland flow outlets |

| Collector and Industrial Roads | 1.0m wide in gutter or 0.10m deep at low point catchbasins | up to crown | 0.10m above crown |
|--------------------------------|--|---|--|
| Local Roads | Not applicable | as required | 0.3m above crown |
| Arterial Roads | 1.0m wide in gutter or 0.10m deep at low point catchbasins | 1 lane clear | up to crown |
| Private Property | minor ponding in swales | no structural damage, ponding in yard areas below building openings - no basement flooding | no structural damage from overland flow |
| Public Property | minor ponding in swales or ditches | no structural damage, ponding in flat areas, no erosion | no structural damage ponding in flat areas, some erosion |

Pre-development peak flows shall be computed by an approved hydrologic model. Watershed definition and predevelopment flows must be approved by the Township.

Preliminary estimates of post-development flow rates may be computed using the Rational Method.

For all systems and for the design of surcharged sewers and detention facilities, the latest version of the computer model OTTHYMO is recommended. Other hydrograph methods may be considered if it is demonstrated that the results are comparable to those from OTTHYMO. Post-development design flows may be determined using the Rational Method only where the design area is less than 40 hectares and runoff control facilities are not considered. The Modified Rational method may be used to model runoff control facilities provided the tributary area to the facility is 2 ha or less.

10.3.1 Open Ditches

In rural areas, industrial areas, or estate residential subdivisions, open ditches may be permitted by the Township. Ditches shall be constructed a maximum of 0.5m and a minimum of 0.15m below the sub-grade of the roadway. Where this cannot be achieved, a sub-drain may be provided under the invert of the ditch and day-lighted once the minimum depth can be attained.

The minimum ditch grade shall be 0.5% and the maximum 5%. In exceptional cases and where ditches are on easements off the Road Allowance, ditches with grades greater than 6% may be allowed by the Township; but these shall be suitably protected against erosion to the satisfaction of the Township. Where ditch grades exceed 5%, the Township may require that an urban cross-section be applied for that road segment.

The minimum ditch protection on all ditches shall be 100mm of topsoil and staked sod on the side slopes and bottom of the ditch regardless of the ditch gradient.

10.3.2 Open Channels

Open channels are to be designed as follows:

- For the Regional Storm where the upstream watershed area exceeds 1 square kilometre;
- For a minimum 25-year return frequency storm with protection from erosion damage for larger storms if required by the Township;
- To maintain the natural storage characteristics of the watercourse;
- To maintain a natural appearance as far as possible; and
- To meet specific requirements of the Conservation Authority in each case.

10.3.3 Sufficient Outlet

All developments must demonstrate that the stormwater generated onsite is directed to a sufficient outlet

A sufficient outlet typically constitutes a lake or permanently flowing watercourse. Sufficient outlet may also include public right-of-way provided that written permission is obtained from the Township. In the case where the discharge is directed over private land, the developer must obtain a legal right of discharge registered on title. Copies of all written documentation such as a legal right of discharge registered on title and/or written permissions from the Township must accompany the design submission.

The designer shall provide calculations for the conveyance capacity of the downstream conveyance routes to demonstrate that upstream, downstream and adjacent landowners do not incur significant adverse impacts, including increased runoff volumes, and that the flow is conveyed in a safe manner.

10.4 STORMWATER MANAGEMENT FACILITIES

All stormwater management facilities are to be designed to ensure that post-development peak flows do not exceed predevelopment levels for storm events up to and including the 1:100-year event. All new stormwater management facilities are required to provide an Enhanced level of water quality protection.

The Township supports the design of stormwater management facilities using hydrologic computer programs including VISUAL OTTHYMO, PCSWMM and SWMHYMO.

All stormwater management facilities are to be designed in accordance with the preferred criteria listed in the Ministry of Environment, Conservation and Parks (MECP) "Stormwater Management Planning and Design Manual and relevant requirements of the Nottawasaga Valley Conservation Authority (NVCA)". The information provided below is intended to supplement that manual.

10.4.1 Maintenance Access

A gravel surface maintenance access is required to the inlet and outlet structures, forebays, sediment drying areas, and emergency spillways. The maintenance access should be looped to provide two access points to all key features. When this



is not possible, dead end access may be provided with a hammerhead turning area of 4m by 17m with a 12m centreline turning radius. The maintenance access must descend to the forebay bottom. Access paths shall not be less than 4m wide and shall be set a minimum of 0.3m above the maximum pond water level.

At locations where overland inlet flow routes or the emergency spillway cross the maintenance access, reinforcing measures shall be incorporated to strengthen the access route.

10.4.2 Emergency Spillway

All stormwater management facilities shall be provided with an emergency spillway designed to convey the greater of the 1:100 year or Regional storm event without failure. The spillway is to be designed with suitable erosion protection given the velocities associated with the uncontrolled flow.

10.4.3 Side Slopes

Stormwater management facilities shall be designed with a minimum side slope of 5:1 above and below the safety shelf. A safety shelf shall be provided with a slope of 7:1 extending 3m horizontally above and below the normal water level.

10.4.4 Sediment Forebay Bottom

The sediment forebay bottom is to be lined with a hard surface material (with consideration of "green" materials) to assist in the maintenance operations of the facility. A Geotechnical Engineer must certify that the forebay bottom design has sufficient bearing capacity to support maintenance equipment assuming the forebay has been drained.

10.4.5 Sediment Drying Areas

Sediment drying areas are to be incorporated into the design of the stormwater management facility such that water from the sediment drains back into the sediment forebay with a slope of 2% to 4%. The sediment drying area shall be designed to accommodate 30% of the forebay volume with a maximum sediment stockpile depth of 1m and maximum side slopes of 4:1. Sediment drying areas should be located in areas that avoid high public visibility, are accessible from the maintenance access road, and are above the 5-year water level.

10.4.6 Signage

Township Standard warning signs shall be provided in clearly visible locations, as approved by the Township.

10.4.7 Landscaping

In cases where there are residential lot(s) adjacent to a stormwater management facility, a minimum 6-meter-wide landscaping buffer with a maximum slope of 5:1 will be required between the maintenance access route and the residential lot(s) to provide separation between the stormwater management facility and private property. This buffer can be included within the stormwater management block.

Refer to Section 14 for additional guidelines related to planting and landscaping for stormwater management facilities.

10.4.8 Operations and Maintenance Manual

A detailed operations and maintenance manual will be required as part of the design of each facility, including inspection checklists, maintenance descriptions and projected frequency, as well as recommendations for facility cleanup.

The certification of a storm water management facility by the developer's engineer will be a requirement of the Certificate of Final Assumption.

10.5 WATER QUALITY TREATMENT UNITS

Where a stormwater management pond is not feasible due to spatial, topographical or drainage outlet constraints, consideration will be given to the use of oil/grit separators or stormwater quality treatment units. The units must be designed to provide 80% TSS removal for 90% of the annual runoff volume for the site using a fine particle size distribution. Oil/grit separators and other stormwater quality treatment units should be used in conjunction with additional quality control measures to provide a treatment train approach.

Supporting calculations and anticipated maintenance requirements shall be provided to the Township along with certification of the design by a Professional Engineer.

Additional documentation may be required on a case-by-case basis.

10.6 EROSION AND SEDIMENT CONTROL

This section presents the minimum criteria to achieve by implementing the design of some commonly used erosion and sediment controls. For specific control measures and/or information on control options that have not been included in this section, other reference resources include the latest revision of the Ministry of Environment, Conservation and Parks (MECP) Stormwater Management Planning and Design Manual; the Nottawasaga Valley Conservation Authority Development Review Guidelines; the Lake Simcoe Region Conservation Authority Watershed Development Policies; and the Greater Golden Horseshoe Area Conservation Authorities Erosion and Sediment Control Guidelines for Urban Construction.

Every control measure and all control plans must meet or exceed the specifications set out in this document and must be designed to achieve satisfactory performance at all times.

The controls should be designed using a phased approach whenever feasible to minimize the exposed area of the site at any given time. There must also be a contingency plan for repair which includes regular inspection and maintenance provisions, particularly following rain events, for the replacement and upgrading of control measures as required to achieve satisfactory performance at all times.

10.6.1 Runoff Control

The development of the control plan shall examine concentrated runoff from adjacent areas that will pass through the site and shall provide for the diversion of the runoff around disturbed areas, in general. If this is not possible, the runoff shall be directed into armoured channels flanked by silt fencing with appropriate low point protection and shall outlet into a capture and treatment facility prior to discharge.



For sites where sediment control ponds are not being proposed, sediment control fences and cut off swales/channels or equivalent control measures shall be placed along all down gradient boundaries of the site.

For sites adjacent to existing residential areas, a cut-off swale/channel shall generally be placed around the entire perimeter of the site to prevent drainage onto private lands. A 3 m wide buffer strip and/or sediment control fence shall be provided along the perimeter of the down gradient boundaries of the site.

10.6.2 Temporary Sediment Control Ponds

Temporary sediment control ponds are generally required for any project that has a construction area greater than 5 hectares, however, may also be appropriate to implement on smaller sites.

The location of the pond(s) shall be that required to intercept runoff from the entire disturbed area unless other controls are implemented in conjunction with the pond, to ensure that satisfactory performance is achieved for the entire area.

In general, a pond shall consist of:

- i. A permanent pool to contain accumulated sediment and post-storm waters;
- ii. A water quality treatment volume that allows for settlement of suspended sediment from storms; and

iii. A forebay to quiet incoming flow if the construction project is anticipated to take place over more than one year.

The required active storage volume shall be designed with a minimum of 125 m3/hectare of contributing area.

The required permanent pool volume shall be designed with a minimum of 125 m3/hectare of contributing area.

The draw down time shall be a minimum of 24 hours, or as required by the governing Conservation Authority.

The outlet works shall consist of a perforated riser system, with a minimum orifice diameter of 75 mm.

The forebay shall be designed with a minimum depth of 1 m, a maximum volume of 20% of the permanent pool, have a stable bottom to allow mechanical clean out, and incorporate sediment depth indicators.

The overall pond shall be designed with a minimum depth of 1.0 m, a maximum depth of 2.5 m, maximum side slopes of 5:1, and an emergency spillway sized to safely convey the 1:100-year storm event from the contributing area.

10.6.3 Silt Fences

Silt fence is to be installed on the perimeter and on the up-gradient side of sensitive areas, streams and rivers, at the base of slopes including those of temporary earth stockpiles. It should not be used in areas of high flows.

When silt fence is proposed, it shall be:

- i. Aligned with site contours;
- ii. A minimum above grade height of 900 mm with a minimum of 300 mm of the fabric toed into the ground.
- iii. Constructed of suitable woven UV stabilized fabric (with a weave density of 270 R or greater) fastened with wire fasteners to 150 mm page wire fencing or acceptable equivalent.

- iv. Supported by steel T bar fence posts with a separation distance of no more than 2.5 m.
- v. Accompanied by a vegetative buffer strip on the down gradient side.
- 10.6.4 Vegetative Buffer Strips

Vegetative Buffer Strips shall be provided between the site alteration area and every down-gradient protected area. They shall be protected from up-gradient erosion by silt fences and shall consist of established vegetation that is growing, whenever possible, on undisturbed soil. They are to be a minimum of 3 m wide between the perimeter of the property and a disturbed area and a minimum of 15 m wide between surface water and a disturbed area. Additional width may be required if the adjacent surface water is classified as a source for cold water fisheries.

10.6.5 Non-Vegetative Buffer Strips

Non-Vegetative Buffer Strips shall be installed where there is not a minimum undisturbed area of established vegetation down gradient of the site alteration area. They shall be a minimum of 2 m wide between the perimeter of the property and a disturbed area and a minimum of 15 m wide between surface water and a disturbed area.

A second silt fence must be installed no closer than 1 m to the primary silt fence and there must be space provided for access to clean out trapped sediment and complete any repairs to the fence.

10.6.6 Channel Low Point Protection (Stone Silt Traps)

Channel Low Point Protection Devices shall be constructed in channels and ditches that will contain concentrated flows to reduce the velocity; thereby reducing erosion of the sides and invert. They shall be designed as follows:

- i. so that the crest of the downstream device is at the same elevation as the downstream base of the device further upstream;
- ii. with 100 mm to 150 mm diameter rip rap, wrapped in geotextile fabric, from the invert of the channel or ditch to a maximum of 250 mm below the top of the channel or ditch;
- iii. with a downstream slope not to exceed 1 vertical in 4 horizontal and an upstream slope not to exceed 1 vertical in 1.5 horizontal; and
- iv. with a 2.5-metre-long excavated sediment trap approximately 600 mm in depth at the upstream face.
- 10.6.7 Construction Access Mats

Construction Access Mats shall be installed at all exits from the site and shall be designed and maintained to remove most of the sediment accumulated on vehicle tires. They shall be designed as follows:

- i. With 300 mm of 100 mm to 200 mm quarry stone placed on a geotextile fabric suitable for allowing ex-filtration of water and preventing the quarry stone from becoming contaminated with the substrate soil.
- ii. To be a minimum of 8 m in width and extend a minimum of 30 m onto the site.



- iii. To be flanked by silt fences and vegetative buffers from the property line to the start of any on-site roadways.
- 10.6.8 Topsoil and Spoil Pile Management

Topsoil and spoil piles shall be designed such that they are not in low areas of a site where water may accumulate and they must be surrounded by one or more silt fences.

Any piles containing more than 100 m³ of material shall be a minimum of 15 metres from a roadway or channel.

If topsoil or spoil piles are to be left in place for more than 45 days, they shall be stabilized by mulching, vegetative cover, tarps or other equivalent means.

Stock piles of any materials are to have a maximum slope of 2:1.

10.6.9 Drain Inlet and Catchbasin Protection

Protection of all potentially affected storm drain inlets and catchbasins shall be accounted for in the design:

Filter cloth protection may be used over the catchbasin inlet where ponding of water will not occur and where traffic will not affect the filter cloth. In all other cases, catchbasin inserts shall be used constructed of filter cloth with a metal support structure.

10.6.10 Site Dewatering

Effluent from site dewatering operations must not discharge directly into receiving bodies of water or streams.

Water pumped from the site shall be treated by control devices such as a sediment control pond, grit chambers, sand filters, pea gravel bags, upflow chambers, swirl concentrators or other appropriate controls, and must not contain particles more than 40 microns in size, or more than 100 mg/L of suspended solids.

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11.1 GENERAL

In any project where sanitary sewers are required, the sewer system is to be designed to carry domestic, commercial, and industrial sewage for the area under consideration. Flow is to be by gravity and pumping will only be considered where other alternatives are not possible and only with written approval from the Township.

If a pumping station concept is approved by the Township, it shall be designed in accordance with the current MECP guidelines and to the satisfaction of the Township.

11.2 CONFIRMATION OF AVAILABLE CAPACITY

The Township maintains a "SewerGEMS" © model of its sanitary sewer systems. Prior to the commencement of any design, the developer and/or designer must contact the Township and confirm that there is adequate capacity to accommodate the proposed project. A sanitary model update will be required for each new development. To facilitate this exercise, the designer shall prepare and provide the proposed development design flows to the Township based on the criteria in this section. The designer shall confirm with the Township whether the sanitary model update will be performed by the Township or is to be done by the designer.

11.3 SERVICE AREA

The system shall be designed to accommodate all on-site drainage areas as well as all external areas to their maximum future development capacity in accordance with the Township's Official Plan.

11.4 DESIGN FLOWS

All sewers shall be sized for peak design flows, including an allowance for infiltration based on the criteria in this section.

11.4.1 RESIDENTIAL

Design Population

In the absence of detailed information, populations should be estimated on drainage areas and proposed land uses identified in the Township's Official Plan or Master Servicing Studies.

When details regarding the proposed land uses are known, the actual number of units shall be used and the design population estimated using the following occupancy values (persons per unit):

Detached, semi-detached = 3.25 ppu

Townhouses, apartments = 2.5 ppu

Average Daily Flow

Average Daily Domestic Flow = 450 L/person/day (excluding extraneous / infiltration flows)



Extraneous Flows

An allowance should be made for the infiltration of groundwater into the sewers and sanitary service connections (infiltration) and for other extraneous water entering the sewers from sources such as maintenance hole covers (inflow). The amount of groundwater leakage into the sewer system will vary with the quality of construction, type of joints, ground conditions, and level of groundwater in relation to pipe. While infiltration can be reduced by proper design, construction, and maintenance, it cannot be completely eliminated and an allowance should be made in the design sewage flows to incorporate this flow component.

A **peak extraneous flow rate of 0.23 L/s/ha** shall be used for sanitary sewer design. This rate assumes strict adherence to construction standards in the installation of sanitary sewers and building connections and does not account for any other extraneous flows such as: foundation drain connections; excessive flooding through maintenance hole covers; significant groundwater problems; etc. Where collection system infrastructure is being designed to convey flows from existing developed areas, the extraneous flow allowance used may be increased based on flow monitoring data and/or system modelling as directed by the Township.

Where a sewer is located within the floodplain of a watercourse, maintenance hole covers are to be raised above the maximum flood elevations, or watertight maintenance hole covers with associated air vents may be required as directed by the Township.

Peak Domestic Flow

Peak domestic flow is to be calculated using the following formula

$$Qp = (P x q x M)/86.4 + I x$$

where;

q = Average daily domestic flow per capita (L/person/day)

M = Peaking factor

I = Peak extraneous flow (L/s/ha)

A = Tributary area (ha)

Peak design flows shall be determined by applying a peaking factor, M, to the average daily flows, based on the Harmon Formula, as follows:

$$M = 1 + 144 + p^{0.5}$$

where; M = Harmon Peaking Factor p = Population (in thousands)

11.4.2 Commercial, Industrial, and Institutional

Equivalent domestic flows for land uses other than residential shall be calculated on an area basis utilizing historical records where possible and / or MECP criteria. Such flows are to be approved in advance by the Township.

11.5 PIPE CAPACITY

The pipe capacity under full flow conditions is to be calculated using Manning's formula, below, and a roughness coefficient of n = 0.013. Sanitary sewers shall be upsized when calculated flows exceed 85% of full flow under future peak flow conditions during inflow and infiltration.

$$Q = 1/n (A) (R^{2/3}) (S^{1/2})$$

where;

Q = Flow capacity of sewer (m^3/s)

R = Hydraulic radius of pipe (m)

- S = Sewer slope (m/m)
- n = Manning roughness coefficient (unitless)
- A = Area (m²)

11.6 VELOCITIES

The minimum velocity in the pipe under full flow conditions is 0.6 m/s.

The maximum velocity in the pipe under full flow conditions is 4.0 m/s.

11.7 PIPE SLOPES

The minimum pipe slope shall not be less than 0.5% or that which is required to meet the minimum velocity.

The first upstream leg of sanitary sewer shall have a slope of not less than 1.0%.

The maximum pipe slope shall be that which is required to not exceed the maximum velocity.

11.8 MINIMUM PIPE SIZE

The minimum pipe size is 200 mm.

11.9 DEPTH OF COVER

A minimum depth of cover of 2.8 metres below the centreline road elevation shall be provided or sufficient depth for basement floor drains and frost cover, as approved by the Township. Where sewers are located within an easement, a minimum frost cover of 1.5 metres may be used, provided that such sewers cross below watermains.

Where sufficient cover does not exist, the Township may consider shallower insulated sewers on an individual and exceptional basis only.

The maximum height of fill is not to exceed OPSD standards unless satisfactory pipe strength design calculations are provided for approval by the Township.

11.10 BEDDING AND BACKFILL

All sanitary sewers are to be installed with bedding and pipe cover (well graded OPSS Granular. 'A' or as recommended and approved by the Geotechnical Engineer and the Township) and backfill in accordance with OPSD 802.010 or 802.030 to 802.032, as applicable. Compaction is to be a minimum of 95% SPD or as indicated in the approved Geotechnical Report. In soft or wet conditions, additional Geotechnical investigation may be necessary to determine the appropriate bedding and backfill measures.

11.11 PIPE CLEARANCES

Minimum horizontal and vertical clearances shall be provided in accordance with MECP guidelines.

11.12 SEWER LAYOUT

Sanitary sewers shall generally be located along the centre of the right-of-way.

Where sanitary sewers are located in easements, the easement width shall be a minimum of 6.0 m.

11.13 MAINTENANCE HOLES

Maintenance holes shall be as per OPSD standards and are to be placed at the beginning and end of each sewer line, at changes in pipe size and/or material, and at changes in grade and/or alignment.

All maintenance holes shall be pre-benched up to or above springline to the satisfaction of the Township.

The maximum spacing between maintenance holes shall be 110 metres for all pipe sizes.

A sufficient drop shall be provided across each maintenance hole to offset any hydraulic losses and in accordance with the following:

| CHANGE IN DIRECTION | MINIMUMM DROP |
|---------------------|---------------|
| Straight run (0°) | 0.03 m |
| 1 - 45° | 0.05 m |
| > 45° | 0.08 m |

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The maximum change in direction permitted in a maintenance hole is 90°.

Drop structures shall be provided when the difference in the inlet and outlet inverts is equal to or greater than 0.9 m that cannot be eliminated by changing sewer grades. Internal drop structures are not permitted.

Safety platforms shall be installed in accordance with OPSD standards for all maintenance hole depths of 5.0 m or greater.

Frost straps are required in accordance with OPSD 701.100.

Obverts of inlet pipes shall not be lower than obverts of outlet pipes. Where maintenance holes are located in areas to be flooded by the major storm design, maintenance hole covers shall be of the sealed variety and properly vented. In all other areas, standard maintenance hole covers shall be used and labelled "Sanitary".

Except for special cases, the downstream pipe diameter shall always be greater than or equal to the upstream pipe diameter. A maintenance hole will be required on the private property side for sanitary services to commercial, industrial, and institutional properties.

Foundation drains shall not be connected to the sanitary sewer.

11.14 SANITARY SERVICE CONNECTIONS

Single connections for residential and industrial use shall be located in accordance with Essa Standard Drawing ESD-1000.02 for each type of residential lot.

Single residential sanitary connections shall be 125mm diameter with a 125mm x 100mm test fitting at property line. For new developments, the service shall be extended 2.0 m beyond property line.

Connections for commercial, industrial and institutional will be considered on an individual basis but will generally be 150mm diameter with a maintenance hole on private property just beyond property line.

The minimum slope for all service connections shall be 2.0%.

Service connections to the sewer main shall be accomplished with a manufactured Tee. Tees shall be installed at a 45° angle to the main, where possible, such that the invert of the tee is at the springline of the main.

Where sanitary main is deeper than 5 m, the use of a sanitary riser is required. The maximum height of riser section should not exceed 5 m.

Service connections shall be located at a minimum depth of cover of 2.6 metres or sufficient depth for basement floor drains and frost cover. Service connections should cross under watermains.

All floor drains are to be connected to the sanitary service connection. Foundation drains and roof water leaders are not to be connected to the sanitary system.

11.15 TESTING

The following testing requirements apply to all new sanitary sewers. Any sections of sewer or service connections which fail to meet the requirements shall be repaired or replaced at the direction of the Township.



Deflection Testing

All newly installed PVC sanitary sewers shall be subjected to deflection testing in accordance with OPSS 410.

Low Air Pressure Testing

All newly installed sanitary sewers shall be subjected to low air pressure testing in accordance with OPSS 410.

CCTV Inspection

All newly installed sanitary sewers shall be subjected to a CCTV Inspection. The sewer and sewer service laterals are again to be subjected to a CCTV inspection prior to final assumption.

Visual Inspection

All maintenance holes shall be visually inspected by the Township for deficiencies.

11.16 PIPE MATERIAL

Refer to Appendix B for a list of approved materials.

12 Water Supply and Distribution System

12.1 GENERAL

The water distribution system is to be designed as a network system to meet the water demand for each area or development under consideration. Long dead-end mains and single supply systems are to be avoided. In order to ensure reliability, a looped system must be provided to the satisfaction of the Township.

All water distribution system components and lubricants are to be as stated on the Township's Approved Products List and must meet the Township's Drinking Water License requirements, applicable AWWA Standards, NSF/ANSI 60 and 61 Standards, and other appropriate standards for approved materials, coatings or linings for distribution system components or chemicals that come into contact with potable water.

Shop Drawings shall be submitted for review and approval by the Township prior to construction for all watermains made of concrete pressure pipe and all chambers, valves, hydrants and other fittings and appurtenances as requested by the Township.

12.2 SERVICE AREA

The system shall be designed to service all areas within the development to their maximum future capacity in accordance with the Township's Official Plan. Allowance shall be made for connection to appropriate watermains in adjacent developments or areas. The exact location for these connections shall be approved by the Township.

12.3 CONFIRMATION OF AVAILABLE CAPACITY

The Township maintains a "WaterGEMS" O model of its water supply and distribution systems. Prior to the commencement of any design, the developer and/or designer must contact the Township and confirm that there is adequate capacity to accommodate the proposed project. A water model update will be required for each new development. To facilitate this exercise, the designer shall prepare and provide the proposed development design flows to the Township based on the criteria in this section. The designer shall confirm with the Township whether the water model update will be performed by the Township or is to be done by the designer.

12.4 DESIGN FLOWS

Watermains shall be designed to meet the greater of the maximum day demand plus fire flows, or peak hour flow, in accordance with the criteria below.

The estimated populations and areas shall be in accordance with the Official Plan.

In general, Fire flows are to be determined based on the most recent publication of the Fire Underwriters Survey of the Insurance Bureau of Canada (FUS) or AWWA Manual M31 – Distribution System Requirements for Fire Protection, but in any case the designer shall satisfy the following at a minimum.

Minimum fire flow:

Residential detached – 100 L/s;

Residential townhouse - 150 L/s;

Industrial and commercial areas – to be determined by the designer in accordance with FUS and / or AWWA requirements and submit for Township approval.

Average daily demand:

450 litres/capita/day

Maximum daily demand factor: determined from the current MECP design guidelines, minimum 1.8 to be used

Pressure reducing valves will be required where water pressures exceed 552 kPa.

12.5 WATERMAIN SIZING

The Hazen-Williams formula shall be used for computing friction losses and subsequently sizing the watermains.

For new mains the Hazen-Williams factor, "C", shall be as follows:

| Watermain F | Pipe C-Factors | * |
|--------------------|---------------------------|-------------|
| | | |
| Pipe Diameter (mm) | "C" Factor | à. |
| · | A supervision . | |
| 150 | 100 | |
| | | sign Na⊷ |
| 200-250 | 110 | <u></u> |
| | | |
| 300-600 | 120 | |
| | | |
| O ver 600 | 130 | |
| | | |
| | * Martin Barris and State | |
| | | |

The minimum size of watermain in residential developments shall be 150mm diameter except for 50mm diameter looped systems in residential cul-de-sacs in accordance with Township standards.

The minimum size of watermain in industrial developments shall be 200mm diameter.

12.6 WATERMAIN PRESSURE

Watermain supply systems are to be designed to achieve the following minimum pressures under the noted demand scenario:

Demand Scenario

Minimum Acceptable Pressure Range

| | <u>kPa (PSI)</u> | |
|--------------------|---------------------|---------------------|
| | Preferred | Accepted |
| Maximum Day Demand | 345 – 485 (50 – 70) | 275 – 550 (40 – 80) |
| Maximum Day + Fire | > 140 (20) | > 140 (20) |
| Peak Hour Demand | 345 – 485 (50 – 70) | 275 - 550 (40 - 80) |

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| Average Day Demand | 345 – 485 (50 – 70) | 275 – 550 (40 – 80) |
|--------------------|---------------------|---------------------|
| Min Hour Demand | 345 - 485 (50 - 70) | 275 – 550 (40 – 80) |

The minimum pressure during the peak hourly demand shall be 275 kPa (40psi).

The maximum pressure under static load or during the minimum hourly demand shall be 550 kPa (80psi).

The minimum pressure when the system is tested under fire flow conditions in conjunction with the design maximum daily demand shall be 140 kPa.

Where water pressure exceeds 620 kPa (90psi), a pressure reducing valve will be required.

12.7 OVERSIZING

Oversizing of watermains will be provided as required to provide for adjacent areas where service is expected to be extended, and to provide fire flow requirements including oversizing of hydrants, openings and leads as required in accordance with modelling results.

12.8 WATERMAIN LAYOUT

Watermains shall generally be located as per the typical road cross-section. Refer to Appendix D, Standard Drawings. When watermains are located in easements, the easement width shall be 6.0m minimum.

12.9 MINIMUM DEPTH OF COVER

A minimum depth of cover of 1.7 metres from finished ground surface to top of watermain is required.

The maximum height of fill is not to exceed OPSD unless pipe strength design calculations are provided for approval by the Township.

12.10 PIPE CLEARANCE

Minimum clearances between other types of underground services shall be provided in accordance with MECP guidelines.

12.11 BEDDING AND BACKFILL

All watermain and appurtenances are to be installed with bedding and backfill in accordance with OPSD to suit applicable pipe material and ground conditions accordingly. Compaction shall be to a minimum of 95% SPD or as indicated in the approved Geotechnical Report.

In soft or wet conditions, additional Geotechnical investigation may be necessary to determine the appropriate bedding and backfill measures.

12.12 RESTRAINING

All joints are to be mechanically restrained in fill areas, as per the manufacturer's recommendations.

Concrete thrust blocks are not permitted. Restrained joints shall be installed at all tees, bends, hydrants, end of mains and connections for all mains. Calculations will be required from the designer in accordance with Township standards and manufacturer's specifications to determine the number of joints to be restrained beyond the bend, fitting, tee etc.

12.13 DEFLECTION OF WATERMAIN

If it is necessary to deflect a watermain to avoid a conflict with other infrastructure, the designer will be required to provide detailed drawings and calculations for approval by the Township.

Joint deflection to enable bends in the watermain is not permitted. All pipe bends or deflections shall be accomplished using manufactured bends.

12.14 SUPPORTING OF WATERMAIN

Where crossing an existing watermain, if more than 0.9 m of the watermain will be exposed, the designer will be required to provide detailed drawings showing how the watermain will be supported during construction for approval by the Township.

12.15 FIRE HYDRANTS

Hydrants shall be installed in accordance with OPSD 1105.010, complete with thrust blocks and mechanical restrainers. Hydrants shall be located as per the typical road cross-section and generally on projections of lot lines, at the end of all culde-sacs, and at dead ends. On rural cross-sections, hydrant access complete with culvert is to be provided. On busy and wide roadways, hydrants may be required on both sides where it may be impractical to run hose lines across

traffic.

The maximum spacing for hydrants shall be a 150 m for low density residential areas and 90 m for high density residential areas, industrial, commercial and institutional land uses or as required by the Township.

Hydrant flanges shall be set such that the final elevation is between 100 mm and 150 mm above finished grade.

In areas where the water table is known to be high, the Township may request that the drain holes of the hydrant be internally plugged.

All hydrants are to be fixed with hydrant markers.

Hydrants are to be flow tested, by an organization approved by the Township, to determine performance while maintaining a minimum residual system pressure of 138 kPa, in accordance with NFPA Standard 291. All hydrants shall be painted yellow with the top and nozzle caps to be provided with two (2) reflective hose connection markers based on the results of the flow testing as follows:



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| Class A | 3785 L/min to 5675 L/min | Green |
|---------|--------------------------|--------|
| Class B | 1900 L/min to 3780 L/min | Orange |
| Class C | < 1900 L/min | Red |

Hydrant numbers will be stencilled on the top of all hydrants by the Township.

Refer to Appendix B for a list of approved materials and specifications.

12.16 VALVES

The size of main line valves shall be equivalent to the size of the watermain.

All valves 450 mm. demater or greater shall be installed in a concrete valve chamber

Valves shall be located as required to meet spacing and intersection requirements described below, or as required by the Township.

Generally, four valves shall be placed at cross-intersections and three valves at tee intersections, such that sections can be isolated without jeopardizing flow to other sections.

Valves shall be spaced such that no more than 40 dwelling units are isolated at one time. In addition, the maximum permissible spacing between valves shall be 150 metres for distribution watermains, and 400 metres for trunk supply watermains.

Air release valves shall be provided at high points of all trunk watermains and, where possible, installed together with valves in valve chambers. Further, where possible, fire hydrants shall be located at all highpoints to minimize the requirements for air release valves.

Drain valves shall be provided at low points of all trunk watermains and, where possible, installed together with valves in valve chambers. All drain valves must have a positive outlet.

The use of pressure reducing valves must be approved by the Township and are to be installed in concrete valve chambers.

12.17 WATER SERVICES

Single 25 mm diameter water services with a curb stop at the property line are to be installed to each residential property. Tracer wire must be installed with and along the pipe and brought to the surface at each service box.

Water services to commercial, industrial, institutional, and multiple-use shall be sized for the specific land use and approved by the Township.

If requested by the Township, a 25mm water service shall be provided to park areas with a non-freeze post hydrant.

Single services for a residential lot shall be located in accordance with Essa Standard Drawing ESD-1000.02. The location of a water service for commercial, institutional, or industrial use will be considered on an individual basis.



Water services shall be located at a minimum depth of cover of 1.7 metres.

Water sampling stations shall be provided where directed by the Township. In general, one sampling station is required for every 300 residential lots. Additional water sampling stations may be required at the Township's discretion.

Fire connections may be required for industrial, commercial or institutional developments.

12.18 WATER METERS

A water meter is required for each water service. The water meter will be sized by the Township using the peak instantaneous flow provided by the designer.

12.19 TRACER WIRE

Where non-metallic pipe (e.g., PVC, PE, concrete pressure pipe) is installed, continuous tracer wire with no joints must be installed with and along the pipe and brought to the surface at each valve box/chamber and hydrant (around port). Where the ends of rolls must be connected, tracer wire connections are to be achieved with a waterproof and corrosion proof connector designed for the purpose and in accordance with the Approved Products List in Appendix ____. Tracer wire is to be attached to the pipe and outside of each valve box by means of tape. See Township standard drawings for additional details. A low frequency continuity test of the tracer wire is to be completed to ensure that the tracer wire has been installed without breaks.

Tracer wire is also being provided with each individual water service in accordance with the Water Services section.

12.20 CATHODIC PROTECTION

Sacrificial caps are to be provided on every **bolt** of all mechanical joints and restrainers. The bolt lengths shall be sufficient to accommodate the caps.

At their discretion, the Township may also require zinc anodes or other corrosion protection measures.

All new metallic watermain fittings, valves, joint restraints, etc. shall be epoxy coated or stainless steel.

All existing metallic fittings, valves and joint restraints must be wrapped end to end with an approved corrosion protection system that includes petrolatum primer (paste), petrolatum moulding mastic, and low temperature petrolatum tape.

12.21 SWABBING

All water servicing greater than 50mm diameter shall be swabbed. Swabs shall be supplied by the Contractor. A swabbing plan and schedule indicating the installation and removal location of the swabs is to be provided to the Township Engineer. Hand swabbing is permitted for small repair sections of watermain.

Swabbing must be completed in the presence of the Township Engineer. All swabs are to be accounted for. The developer/contractor will be responsible to locate any swabs not accounted for.

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The following testing requirements apply to all new sections of watermain. Any sections which fail to meet the requirements shall be repaired or replaced at the direction of the Township.

A temporary bypass connection is to be installed to permit testing of the system.

12.22.1 Hydrostatic Testing

The contractor/ developer shall supply all materials, labour and equipment necessary to complete a hydrostatic leakage test. All hydrostatic leakage testing procedures are to be witnessed by the Township Engineer.

Pressure and leakage test shall be performed on the completed distribution system. The distribution system shall be tested in accordance with AWWA specification C600-93 or the latest revision thereof. Test sections are not to exceed 500m in length unless approved by the Township Engineer.

12.22.2 Disinfection

The contractor/ developer shall not operate any main valves, blow-offs, by passes during the period of chlorination. All testing procedures must be completed in the presence of the Township Engineer.

All chlorinating procedures are to be completed in accordance with the Watermain Disinfection Procedure - August 1, 2020 from the Ministry of the Environment, Conservation and Parks and AWWA standard C651-14 or the latest revisions thereof, as applicable. A minimum initial chlorine concentration of 50 mg/l shall be provided to the system and checked by the Township Engineer. Following a minimum 24 hours, the system is to be tested for residual chlorine prior to flushing. Prior to flushing the system, the discharged chlorinated water must be dechlorinated utilizing a neutralizing chemical. Flushing of the watermain is not to proceed until all safety precautions have been completed to the satisfaction of the Township Engineer.

12.22.3 Bacteriological Testing

Sampling of the water system is to be completed by the Township Engineers and the Township's operating authority. 25mm tails at locations acceptable to the Township Engineer, are to be installed by the developer/contractor for the purpose of sampling. The new water system shall be deemed acceptable for consumption following two (2) consecutive acceptable potability test results in accordance with AWWA C651-14. For a test result to be deemed acceptable, the bacterial analysis of the sampled water must indicate a Total Coliform count of 0 CPU's per 100ml, an E.Coli count of 0 CPU's per 100ml and Background count of 0 CPU's per-100ml.

Should the results of the Bacterial Analysis of Water be deemed unacceptable, additional swabbing, chlorinating and flushing and sampling will be required.

Following the confirmation of acceptable sampling results, valves are to be checked for operation by the Township's operating authority and service connecting curb stops shall be checked for operation by the developer's engineer.

12.22.4 Continuity Testing

A continuity test of the tracer wire shall be completed utilizing low frequency testing methodology with point-to-point readings recorded and approved by the Township.

12.22.5 Final Connection

The final connection shall not be made until all of the above testing has been satisfactorily completed and the Township has provided written notification to proceed.

The length of pipe required to connect the new water system to the existing distribution system shall not exceed 5.5 metres and shall be disinfected prior to installation in accordance with AWWA C651.

The completion of the final connection to the existing water system may only proceed upon written direction from the Township Engineer.

12.23 MATERIALS

Refer to Appendix "B" for a list of approved materials and specifications

13 Grading and Drainage

13.1 GENERAL GRADING

For residential lots, it is preferable to have one consistent slope of between 2% - 5% for the entire rear yard area; however, where this is not possible, lots are to be provided with a rear yard amenity area in which a slope of between 2 - 5% is provided for a minimum of 5 m or 75% of the average rear yard length, whichever is greater. Slopes from the end of the rear yard amenity area to the rear lot line shall not exceed 3:1. All exceptions must receive written approval from the Township.

All other areas shall have a minimum slope of 2% and a maximum slope of 3:1, to a maximum vertical difference of 1.0 m. Where the vertical difference is greater than 1.0 m, a retaining wall system shall be implemented.

Any lot with a 10% average grade may require a split-level dwelling and cross sections may be required.

13.2 SWALES

All standard yard drainage swales are to be designed with a minimum slope of 2%, a maximum slope of 5%, and a maximum side slope of 3:1.

An exception to the minimum slope criteria may be where enhanced drainage swales have been designed in accordance with a storm water management plan for the subdivision which has received prior approval from the Township.

All standard yard drainage swales shall have a minimum depth of 0.15 m. The maximum depth for side yard and rear yard swales shall be 0.3 m and 0.4 m respectively.

The maximum contributing area to a side yard swale shall be 500 m2. For rear yard swales, the maximum contributing area shall be 1000 m2 and the maximum length shall be 60 m. The flows from the contributing areas should be calculated and the capacity of the swale confirmed to ensure adequate conveyance.

13.3

REAR YARD CATCHBASINS

The use of rear yard catchbasins (RYCB's) are highly discouraged and the designer should design the overall subdivision grading in order to avoid their use. Designs using RYCB's require special approval from the Township and will only be considered where there is no feasible alternative. If RYCB's are proven to be necessary, overland grading shall be designed such that all structures, dwelling and ancillary, are protected from flooding if the inlet is blocked or surcharged by a major storm event.

Where a RYCB lead extends between houses, the RYCB shall be placed equally between adjacent lots, straddling the shared lot line, and within a minimum 6.0 m wide easement which extends from the rear lot line to the fronting street property line. The 6.0 m wide easement shall be conveyed to the Township. The catchbasin lateral shall be placed such that the center is 0.5 metres off of the common lot line. The catchbasin shall be placed such that the center is 1.5 m offset from the rear lot line unless rear yard sloping requires a greater offset. The maximum contributing area to the inlet shall be 2,500m2.

Storm sewers located in side-yard easements shall be offset from property line as required to avoid damage to storm pipe during the installation of lot line fencing. Storm sewers shall be designed and installed to achieve minimum 1.2 m cover and to ensure adjacent house footings will have no influence loading upon the storm sewer. The designer may be requested to demonstrate the influence of footing loading with appropriate cross-sections.

Where the RYCB lead extends across the rear yard of a corner lot, the RYCB shall be placed on a minimum 6.0 m wide easement with the catchbasin placed such that the center is 1.5 m offset from the rear lot line. The maximum contributing area to the inlet shall be 2,500 m2.

All RYCB leads are to be concrete and a minimum of 300 mm in diameter.

13.4 DRIVEWAYS

The minimum slope on any driveway shall be 2%. For residential lots, this minimum shall be measured from the garage sill to the property line, as well as on the driveway apron, being the area within the municipal road allowance. The preferred maximum slope for any driveway is 5%, however an absolute maximum of 7% may be permitted where necessary.

14 Streetscape / Parkland Trees

14.1 TREE PRESERVATION

This section provides policies, general standards and guidelines for the protection of existing trees during construction and for the planting of new trees. The section outlines tree inventory, preservation and protection requirements to ensure existing vegetation is retained where possible, and measures to ensure damage to existing vegetation is minimized.

14.1.1 Tree Inventory Requirements

For the entire property to be developed, the Proponent shall submit a tree inventory drawing for all trees on the subject property as well as for any trees on adjacent property whose critical root zones extend onto the property to be developed.

Where there is more than one tree to be removed, the trees must be numbered. These numbers must correspond to an arborist report and the Tree Protection plan as outlined below

A report prepared by an arborist or landscape architect which details specific and accurate information about trees that may be impacted by the development initiative and identifies the nature of the work to be undertaken and appropriate protection measures. Information on the report may include location, species, size, condition, structural integrity, disease, infestations and vigour. This report helps Township Staff to determine the legitimacy of tree removal requests by summarizing the actions necessary, and results in the efficient processing of inspection requests and applications. Arborists undertaking reports must be certified with the International Society of Arboriculture (I:S.A.).

As required to control the proliferation of the Emerald Ash Borer, ash trees will not be permitted as nursery tree plantings.

A Tree Protection Plan shall be submitted, designating trees to be preserved and trees to be removed on the subject and adjacent property. Replacement trees for those removed should also be indicated and clearly summarized in a tabular chart directly on the Plan.

14.1.2 7

2 Tree Protection Measures and Techniques

Tree protection fencing shall be erected at the critical root zone or beyond, prior to commencement of any clearing, grading or other construction activity.

Tree protection shall not be removed until completion of all construction activity.

The minimum required distances for determining a Tree Protection Zone (TPZ) is as per the following Table. Some trees depending on health, the uniqueness of the species, soil type and competition from other trees may require larger TPZ at the discretion of the Township.

| TRUNK DIAMETER (DBH)* (CM) | MINIMUM PROTECTION DISTANCES REQUIRED (M)** |
|----------------------------|---|
| <10 | 1.8 |
| 11-40 | 2.4 |
| 41-50 | 3.0 |
| 51-60 | 3.6 |
| 61-70 | 4.2 |
| 71-80 | 4.8 |
| 81-90 | 5.4 |
| 91-100 | 6.0 |

Table 7: Tree Protection Distances

* Diameter at breast height (DBH) measurement of tree trunk taken at 1.4 metres above the ground.

** Tree Protection Zone distances are to be measured from the outside edge of the tree base.

14.1.3 Tree Protection Barriers

Tree protection barriers, erected for trees and tree groupings situated on the Township road allowance, parkland, and open spaces must be maintained. Tree protection barriers are to be constructed of steel T-bar posts (50 mm x 100 mm x 1.8 m) spaced 3 m, and spray painted with fluorescent orange paint. Livestock fencing and filter fabric is to be attached to the posts. All supports and bracing used to secure the barrier should be located outside the Tree Protection Zone (TPZ) to minimize damage to roots.

Where protection barrier is on slopes greater than 4%, provide a ditch on the elevated side of the fence, lined with filter fabric and backfill to prevent erosion.

14.2 LANDSCAPE REQUIREMENTS

This section provides guidelines, policies of procedure and practice that are to be adopted by the proponent. Specific guidelines are included for plant materials, including nursery grown trees, topsoil, shrubs, groundcovers, sod and seed mixtures.

14.2.1 Species and Size of Plant Materials

Suitable trees, varieties, and cultivars to be planted in Essa are to be approved by the Township.

Caliper size shall be determined in accordance with the Canadian Standards for Nursery Stock and measured at 150 mm above grade for trees 40 - 100 mm caliper and at 300 mm above grade for trees larger than 100 mm caliper.

The minimum size for trees and shrubs shall be as follows:

| Deciduous trees | 50mm caliper | |
|---------------------------------|---------------------|--|
| Coniferous trees | 1.5m height (min.) | |
| Deciduous shrubs | 60cm in height | |
| Evergreen shrubs (non-columnar) | 60cm in spread | |
| Columnar evergreen shrubs | 100-120cm in height | |

14.2.2 Diversity

The Township does not support the planting of monocultures (the predominance of one tree species). This section supports our commitment to maintain tree health and bio-diversity, and recognizes the importance of a proactive integrated pest management approach to street tree planting.

As such, individual species should not occur in like rows in excess of 3 trees. The Township encourages a randomized approach to sequencing of tree species. This allows flexibility in terms of soil types and adjacent urban design.

14.2.3 Topsoil & Seed Requirements

14.2.3.1 Topsoil

The stockpiled topsoil shall be spread over the sub-grade to a minimum depth of 200mm. It shall be free of stones and roots over 25mm in diameter and other extraneous matter. All topsoil may be subjected to a physical and chemical analysis by a qualified laboratory to determine its suitability for the intended purpose.

Where the developer is required to supply additional topsoil to meet minimum requirements, it shall be fertile, friable natural light organic loam and shall be approved by the Township before placement.

The developer must supply all necessary fertilizers to eliminate any chemical deficiencies as indicated by a soil analysis conducted by the developer.

Such an analysis shall be completed by a recognized laboratory specializing in soil analysis and all associated cost shall be at the expense of the developer.

15.2.3.2 Native Topsoil

Use native topsoil, stripped and stockpiled, provided it meets specified requirements.

If native topsoil is inadequate in quantity, quality or both, use imported topsoil meeting specified requirements for balance of topsoil required.

15.2.3.3 Topsoil Stockpiles

Control and eliminate perennial grass and noxious weeds including their root systems until stockpile topsoil is required for landscape use. Stockpile topsoil shall be reasonably free of all weed growth before placement on site.

Perform weed control, when necessary, in accordance with relevant government chemical pesticide application legislation . Screen stockpiled topsoil, as necessary. Use a screener having a wire mesh screen size opening of minimum 25mm and maximum 50mm to remove stones, soil lumps, foreign material, debris, undesirable plants and roots.

Topsoil Stockpiles are not to be located in open space or park areas.

15.2.3.4 Preparation of Subgrade

Loosen subgrade by scarifying or tilling using discs, harrows or other suitable equipment to a depth 75-100mm immediately before placing any topsoil. Repeat cultivation in areas where equipment used for hauling and spreading topsoil has compacted topsoil.

15.2.3.5 Preparation of Final Grade

Rake, chain drag and lightly roll top soiled areas, remove all ridges and fill all depressions. On larger areas, use hydraulic power box rake or similar mechanical equipment to: remove soil lumps, rocks and debris; fill and level low areas; and correct other grades deficiencies in preparation of seed or sod bed.

14.2.4 Plant Material Considerations

15.2.4.1 Stock

High quality nursery stock is a prerequisite to ensuring the survival of trees. As such, all trees shall conform to the Canadian Standards for Nursery Stock and be of standards quality, true to name and type, and a first-class representative of their species or variety.

Balled and burlap wrapped 50-mm caliper stock is the Township of Essa's minimum requirement standard.

Whips (bare-root stocks measuring 1-1.5m in height) may be planted in low-profile, rural areas, and stormwater management facilities and seedlings or whips may be used for naturalization or restoration areas.

15.2.4.2 Form Vigour of Trees

All trees shall be true to type, structurally sound with no evidence of dead branches, sun scald, frost cracks, abraded or broken bark, and be free of insect or disease infestation.

All trees shall have a full, well-developed crown with one distinctive vertical leader, and a root system typical of the species. All parts shall be moist and show active green cambium when cut.

All trees must meet these specifications at the time of planting and final inspection by the Township's designate. Trees that are rejected will be replaced at the Proponent's expense.

15.2.4.3 Planting Holes, Drainage and Root Pruning – Balled and Burlapped ("B&B") Trees

Planting in linear trenches (min. 1.2 width) along street boulevards is strongly encouraged.

Where planting in trenches is not feasible, the planting hole must be at 400cm greater than the diameter of the rootball; the depth of the hole will be the same as the depth of the ball.

15.2.4.4 Backfilling

Backfill is to be placed in layers approximately 15cm in depth, and firmly tamped in place in such a manner that the tree retains its vertical position without support. Particular care is to be taken to ensure that no air pockets remain under or around roots and that damage does not occur to the root system. The fill shall be thoroughly watered immediately after planting.

At grade, a ridge of soil located at the edge of planting hole shall be formed to a height of 10cm, to act as a catch basin for any subsequent watering and to retain mulch.

B&B trees shall have the burlap cut and rolled back from the ball and buried under least 10cm of soil. All wire, rope, burlap and twine from around the top 1/3 of the root ball shall be cut and removed.

All excess fill removed from the planting holes, which does not meet planting specifications, shall be removed by the Proponent, along with all branches, roots from pruning, leftover wire and plastic shields from staking. Any damage done to this surrounding site shall be repaired by to the Proponent at their own cost.

15.2.4.5 Bed Preparation

Within 48 hours of planting, a circular (or other such suitable shape as may fit the site) area around the root ball shall be cultivated to improve future root growth.

The area shall extend to a minimum of 1m from the trunk of the tree in all directions; all sod shall be removed from the area, and the soil shall be broken up and cultivated to a depth of at least 15cm within the 1m radius.

15.2.4.6 Pruning

The crown of the tree shall be pruned from the bottom up at the time of planting to remove all dead and damaged branches. Branches with included bark should be pruned from the tree.

The terminal or leader is not to be pruned unless broken. All cuts shall be collar cut as per ISA standards.

On all cuts over 2cm in diameter and bruises or scars on the bark, the injured cambium shall be cut back to living tissue and removed. Wounds shall be smoothed and shaped so as not to retain water. Large wounds produced by any means other than branch pruning may render the tree unacceptable, requiring replacement.

15.2.4.7 Staking

Where staking is required, B&B trees shall be supported by two (2) steel t-bars (50 mm x 100 mm x 2.4m in length) driven outside the ball in line with the direction of the prevailing wind. The stake must be driven at least 90cm below grade line,



leaving at least 5cm between the top of the stakes and the first branch. Stake replacement shall be such that no main roots are severed by the stake being driven into the ground.

Within storm water facilities, staking is not required. However, should any trees move in either direction 10% or more from the vertical plane, the Proponent is responsible for straightening and staking the trees within one week of notification from the Township.

The Proponent will be responsible for the removal of stakes prior to final inspection.

15.2.4.8 Tree Ties

Ties shall be made from burlap ranging in width from 25 – 50 mm, which must completely encircle the trunk. Plastic hoses and rubber ties are not accepted.

For B&B trees where the two stakes are driven into the ground outside the root ball, the tie tension must be such that the tree is firmly, but not too tightly, supported and remaining in a vertical position.

On final inspection of the guarantee (after spring flush), the Proponent will be responsible for the removal of the ties.

15.2.4.9 Mulching

Within 48 hours of planting, a mulch comprised of wood chips or shredded bark, shall be spread over the entire planting pit and the bed preparation to a surface diameter of 2 m.

All shrub planting beds are to be mulched to the same depth of 75mm after settlement.

Mulching is to be topped up prior to final inspection.

15.2.4.10 Minimum Root Ball Diameter

The minimum acceptable root ball diameter for B&B trees shall be:

- 55cm for 40mm caliper trees
- 65cm for 50mm caliper trees
- 85cm for 75mm caliper trees
- 100cm for 100mm caliper trees
- 165cm for 150mm caliper trees

15.2.4.11 Watering

The Proponent shall apply water at low pressure underground. The amount of water per tree shall be 40 litres or until excess water runs off. Watering should be applied directly after planting. The initial application of water should be included in the planting price.

After the initial application of water, the watering and maintenance plan should be followed to ensure the health of the tree.



15.2.4.12Rodent Guards

In some locations, such as naturalized areas where grass is permitted to grow quite long, rodent guards may be required to protect the trees from girdling by rodents.

Rodent guards shall consist of plastic spiral wrap wound around the base of trunk, up to a height of 60cm. These guards must be installed after planting and inspection and then removed after two (2) years of growth.

14.2.5 Acceptance and Warranty

All plant materials shall be maintained by the Proponent, from the time of planting until acceptance by the Township.

A maintenance regime shall consist of proper cultivation, weeding, watering and pruning to establish and maintain plant material in a healthy growing condition.

The Proponent shall arrange a time to have the plant material accepted following the guarantee period. At the time of inspection, all beds and tree pits shall be freshly cultivated, weeded and in a tidy condition, with all rubbish leaves and dead plant debris removed.

14.3 DESIGN CONSIDERATIONS

In this section the requirements relative to acceptable tree species and related landscaping components with respect to specific applications are provided.

14.3.1 Acceptable Trees

Proposed tree species are to be provided to the Township for approval.

In selecting trees for specific applications, consider the following factors are to be considered:

- i. Assess conflicts with power lines, sidewalks and underground utilities.
- ii. Select of large trees where space is available to maximize planting effect.
- iii. **Encourage** diversity of species to reduce susceptibility to disease.
- iv. Utilize native species as much as possible.
- v. Consider long-term maintenance pruning requirements of trees.
- vi. Select species with respect to soil microclimate requirements.
- vii. Select species with respect to salt tolerance and specific moisture requirements.

14.3.2 Mixture and Proportion of Species

The planting of monocultures (dependence on one plant species) within the streetscape is detrimental to the maintenance of tree health and increases a tree's overall susceptibility to pests and diseases.

Accordingly, the Township of Essa encourages biodiversity in the streetscape by the following measures:

- i. The random sequencing of tree species is encouraged within a planting scheme.
- ii. The planting of any one individual species is limited to eight (3) consecutive trees in any given planting scheme.
- iii. To ensure diversity, the amount of one tree genus shall not exceed more than 20% of all plantings, and the amount of one tree species shall not exceed more than 10% of all plantings.

14.3.3 Tree Siting and Setback Requirements

A minimum of one (1) tree, planted in the boulevard, is required per residential lot unless determined otherwise. The minimum spacing of trees along the lot flankage on a side street shall be 12m. When planting near utilities, the location and species of the plant material must be coordinated with mechanical and electrical site plans to ensure the non-obstruction of storm and sanitary sewers, water service, hydro, telephone, gas lines and other services. All utilities must be demarcated by the Proponent prior to any excavation.

In order to reduce conflicts with utilities, driveways, curbs, sidewalks and other trees, the following setbacks shall be incorporated:

- i. Large crown trees are to be planted at intervals ranging from 15 to 18 metres apart. Small crown trees are to be planted between 6 to 8 metres apart.
- In single family detached residential developments as well as multi-unit developments, a minimum of one (1) shade tree per lot is required.
- iii. The minimum spacing for trees along a lot flankage on a side street is to be 12m.

14.3.4 Park and Playground Requirements

- 15.3.4.1 Minimum Park Requirements for Residential Developments
 - Components of Parkland, and other Open Space considerations are as follows:
 - i. The Township may require services to parkland parcels, as follows: 25mm water service, 125 mm sanitary sewer service, 250mm storm sewer service and a hydro service as minimum requirements.
 - ii. Park grading: minimum slopes of 1.5 2% graded to a manner to maintain proper drainage
 - iii. Park fencing: 1.5 m high chain link fence
 - iv. Park turf: The minimum requirement for park turf may be established utilizing a seed mix conforming to the landscape architect's specifications. Once stablished, the turf must have a consistent sod like appearance.
 - v. Development construction: undeveloped, disturbed blocks shall be graded, seeded, and maintained by the Proponent until construction commences thereon.



Where development abuts a Park or Open Space, runoff from the development property shall not drain into the Park or Open Space. A rear lot catch basin may be required.

15.3.4.3 Park and Open Space Features

Required park and open space features include:

- i. Chain link fence (1.5m) to Township standards where Park property is adjacent to private lots. No gates into parks, open spaces, or stormwater management facilities are permitted from private property. Park frontage on streets is not fenced.
- ii. Park walkways that are to be maintained in the winter must be 3m width, asphalt surfaced. Otherwise, park walkway material is stone dust.
- iii. Playgrounds must conform to the latest Canadian Standards Association (CSA) standards for play spaces and equipment.
- iv. At least one (1) light standard must be provided at playgrounds for security.
- v. Playgrounds and walkways must conform to AODA standards in addition to CSA standards.
- vi. A park sign indicating the name of the park is to be provided by and installed by the developer at a location approved by the Township. Specifications for the park signs are to be submitted, reviewed and approved by the Township.

15.3.4.4 Parking Supply Standards

Parking lots may be required for Neighbourhood Parks at the discretion of the Township.

Parking space requirements for Community or Township parks will be determined on a case-by-case basis.

15.3.4.5 Park Development Standards

All Parks and Open Space developed in the Township of Essa shall comply with the following standards:

Can/CSA Standard Z614 – Children's Playspaces and Equipment (latest edition).

Accessibility for Ontarians with Disabilities Act (AODA), including all associated Regulations. Refer to US Guide to ADA Guidelines for Accessible Play Areas for clarification and requirements of "accessible" play features.



Appendix B: Approved Products List

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Appendix C: Traffic Impact Assessment Guidelines



TRAFFIC IMPACT ASSESSMENT GUIDELINES DOCUMENT

June 2022

Introduction

The intent of these guidelines is to provide a clear and consistent approach to be applied by Township staff and followed by developers with respect to traffic impact assessment requirements within the context of the site plan application process.

It is typical that a traffic impact study be required when:

- the proposed development will generate 100 or more vehicle trips during the peak hour (peak hour of the road or peak hour of the development); or
- the proposed development will generate less than 100 vehicle trips during the peak hour, however, the site or study area characteristics are such that traffic or other transportation issues or concerns may have to be addressed to accommodate the development.

As stated by ITE¹: This site trip generation threshold is appropriate for the following reasons:

- an additional 100 vehicles per hour can change the level of service or appreciably increase the volumeto-capacity ratio of an intersection approach; and
- left or right-turn lanes may be needed to satisfactorily accommodate site traffic without adversely impacting through (non-site) traffic.

¹ Transportation Impact Analyses for Site Development, An ITE Proposed Recommended Practice. ITE, 2005.



For smaller scale developments (i.e. less than 100 trips during the peak hours) where little or no impact to the external road system is anticipated, a traffic study of reduced scope is typically accepted.

Terminology

There are several names given to studies that assess the potential traffic impacts of future development (i.e. traffic impact assessment, traffic impact study, traffic review, traffic brief, traffic statement, etc.). For the purpose of clarity, the following terminology has been considered in developing the traffic impact assessment guidelines for the Township:

| Traffic Impact Assessment | Refers to the generic requirement that an assessment be completed to address the potential traffic impacts of a proposed development on the adjacent road network. The traffic impact assessment will be in the form of a <i>Traffic Impact Brief</i> or <i>Traffic Impact Study</i> , whichever is deemed appropriate based on the thresholds contained herein and in consultation with Township staff. The Traffic Impact Assessment Guidelines, as recommended in this review, are intended to provide guidance to the Township and developer with respect to establishing the appropriate scope of traffic impact |
|------------------------------|---|
| | assessment required (i.e. Traffic Impact Brief or Traffic Impact Study). |

- Traffic ImpactThis is a reduced scope traffic impact assessment, usually required for small scale
developments where little or no impact to the adjacent road network is anticipated.
- Traffic ImpactThis is a comprehensive traffic impact assessment, usually required for medium and
large scale development where greater impacts to the adjacent road network are
anticipated.

The thresholds and content requirements for a Traffic Impact Brief and Traffic Impact Study are detailed in the Traffic Impact Assessment Guidelines provided herein.

MTO and/or Municipal Requirements

Should a development have frontage and/or access to a Provincial highway or County road, or be considered a major traffic generator in close proximity to a Provincial highway, consultation with the Ministry of Transportation of Ontario and/or the County is recommended to confirm their requirements with respect to a Traffic Impact Assessment. While the Township's Traffic Impact Assessment Guidelines are generally consistent with the requirements of MTO, the applicant must confirm the requirement of all relevant road authorities. Regardless, it is expected that developments impacting Township's roads will submit a Traffic Impact Assessment that satisfies the requirements noted herein.



Purpose of a Traffic Impact Assessment

The purpose of a Traffic Impact Assessment, regardless of scope, is to examine and document the potential impact of traffic generated by a new use, and to identify any mitigating measures/improvements that may be required to accommodate the additional traffic volumes. Depending on the required scope, the assessment may be limited to the proposed access, or expanded to include impacts at nearby intersections, interchanges and adjacent road sections. The type of Traffic Impact Assessment required, be it a Traffic Impact Brief or Traffic Impact Study, and content to be included shall be determined based on the thresholds noted below and in consultation with Township staff.

Traffic Impact Assessment Thresholds

The following thresholds are to be considered by applicants as a general rule. Prior to undertaking a Traffic Impact Assessment, the applicant must confirm the appropriate level of assessment through consultation with Township staff.

- Traffic Impact
BriefFor developments expected to generate less than 100 vehicle trips during the peak
hours (total of inbound and outbound trips), a Traffic Impact Brief may be considered,
provided that the site or study area characteristics are not such that would otherwise
indicate that traffic or other transportation issues/concerns are likely to arise given
the proposed development.
- Traffic ImpactFor developments expected to generate 100 or more vehicle trips during the peak
hours (total of inbound and outbound trips), a Traffic Impact Study is required for
review and approval by the Township.

A Traffic Impact Study may also be required for a development that will generate less than 100 vehicle trips during the peak hour, if the site or study area characteristics are such that traffic or other transportation issues or concerns may have to be addressed to accommodate the development.

Traffic Impact Brief - Scope of Work

A Traffic Impact Brief has a reduced scope in context of the anticipated trip generation of the development, and hence reduced potential for impacts that may result. A Traffic Impact Brief should address the following:

1. Study Area: Inventory the existing road system to which the development fronts and/or in the immediate area of the development. This should include the number of lanes, speed limits, urban/rural cross-sections, sidewalk locations, pedestrian crossings, etc. The study area is to be confirmed with the Township prior to commencing the assessment.

- 2. Proposed Development: Detail the proposed land use and phasing/completion year. Provide an overall plan illustrating the proposed access locations, the internal road network, building locations, parking, etc. as applicable.
- 3. Site Traffic: Based on the land uses, determine trip estimates for the most appropriate peak hours. Typically, these will be the weekday AM and PM peak hours of the adjacent road, although for commercial, entertainment, religious, institutional or sport facility uses, a weekend peak may be more appropriate. For uses that are expected to experience seasonal variations, the peak seasonal conditions should be considered. The trip estimates should be based on published trip rates, data from other representative sites, or based on sound engineering ("First Principles").
- 4. Site Access Assessment: The site access should be reviewed in consideration of the volume and type of vehicles to be generated by the site, and the proposed access location and configuration. The site access assessment should consider the Township's standards as outlined in Township of Essa Engineering Design Standards and Specifications Manual.
- 5. Sight Line Assessment: At each access point, the sight distance requirements should be determined based on the Township's guidelines (as detailed in By-Law No. 2015-11 regulating entrances to Township roads), and the availability of appropriate sight lines determined from actual field measurements or engineering plans.
- 6. Site Plan Review: On-site circulation should be reviewed in context of the volume and type of vehicles to be served (including emergency vehicles, garbage collection or other appropriate design vehicles) and the ability of the site to accommodate site traffic without impact to the adjacent road system. Circulation of pedestrians and cyclists should also be considered as well the potential impact the development may have on existing pedestrian/cycling facilities. Queuing assessments may be required for land uses that induce on-site queues (i.e. restaurants with drive thru facilities, gas stations, drive-thru bank machines, car washes, etc.) The intent of a queueing assessment is to ensure that on-site queues can be accommodated wholly within the site without causing disruption to the operations of the adjacent road system (i.e. queue lengths that block access points or spill back onto the adjacent road.
- 7. Documentation: A report should be prepared to detail the findings related to the above. Relevant maps, graphs, tables and figures should be provided. The Traffic Impact Brief should be able to provide a conclusive statement that the proposed development will not adversely affect the operations or the safety of the adjacent road system (or conversely, identify any improvements that may be required to accommodate the development), and that the development plan (including the access, parking area and internal circulatory system) is adequate to suit the site traffic.

Traffic Impact Study - Scope of Work

A Traffic Impact Study should address the following:

1. Study Area: The study area should extend far enough, within reason, to contain all municipal and provincial roads that will be noticeably affected by the traffic volumes to be generated by the proposed development. The study area is to be confirmed with the Township prior to commencing the assessment. The Township reserves the right to establish the study area as may be deemed necessary.

A description of the existing transportation system in the study area, using a combination of maps and other documentation, should identify relevant information, such as:

- existing roads, number of lanes, on-street bike lanes, posted speed limits, sidewalk locations, pedestrian crossings;
- the lane configurations and control of the key intersections/roundabouts to be considered in the study;
- the provision of on-street parking in the vicinity of the development site and the potential for this to impact key intersections being analyzed;
- other traffic controls and transportation facilities; and
- other features of interest such as designated trails, walkways, etc.
- 2. Horizon Years & Periods of Analysis: The horizon years should coincide to the anticipated full build-out of the site, in addition to 5 and 10 years beyond full build-out. Interim horizon years must also be considered as necessary for the staged implementation of the identified road system improvements.

Typically, the weekday AM and PM peak hours of the adjacent street will constitute the peak hours to be addressed. However, in the case of commercial, entertainment, religious, institutional or sport facility use, the weekend peak hours may be more appropriate. During the initial consultation process with the Township, the Consultant should determine the selected peak periods for analysis and identify an appropriate method for considering peak seasonal conditions.

3. Existing Traffic Volumes: Figures illustrating the existing traffic volumes and turning movements for the study area roads and intersections are required. Traffic volumes may be acquired from the Township, County, MTO, from other transportation studies in the area, or through new traffic counts. To ensure representation of existing conditions, traffic count data should be no more than 3 years old. Ideally, traffic counts will be conducted so as to capture peak seasonal conditions; however, where such data capture is not feasible due to timing constraints, a reasonable adjustment to account for peak seasonal variations should be established and verified by the Township and applied to the existing traffic volumes.

4. Future Road Network & Background Volumes: Future transportation improvements to the study area road network that are currently being considered should be identified. These improvements should be described to a level of detail sufficient to assess their implications for travel to/from the development site. In each case, identify the status and anticipated date of implementation for consideration in the Future Background and Future Total traffic conditions.

Future traffic volumes should be prepared for each horizon year, with consideration for annual growth rates to reflect general growth in traffic in addition to the growth specific to new development.

General growth can be estimated based on historic growth in area traffic volumes, and/or future projections of area population and employment. Consideration can also be given to other area transportation studies.

All significant developments under construction, approved or in the approval process and that are likely to contribute additional traffic volumes to the study area within the study horizon period, should be identified. The trips that are expected to be generated by these developments should be included in the future background volumes (trips can be established from a corresponding Traffic Impact Study or based on the land uses and published trip generation rates).

- 5. Proposed Development: The Traffic Impact Study should provide a full description of the proposed development, including the following as applicable:
 - municipal address;
 - existing and proposed land uses;
 - total development area and summary by use (e.g. type and number of residential units, commercial gross floor area, etc.);
 - anticipated tenants;
 - planned phasing and build-out period.

A site plan, concept plan, or similar, is to be included in the Traffic Impact Study document. If the proposed development is to be constructed in phases, describe each phase and the proposed implementation timing for each.

- 6. Site Traffic Trip Generation: Trip generation estimates are to be based on the proposed land uses, size of development and trip generation data from:
 - the ITE Trip Generation Manual (most recent edition);
 - trip generation surveys conducted at similar developments/proxy sites; or
 - "First Principles" calculations of anticipated trips to/from the site.

Where appropriate, it may be justified to reduce the base trip generation estimates to account for passby trips and/or internal/shared trips (in the case of a multi-use development). The resulting estimates are referred to as "new" vehicle trips. All trip generation assumptions employed in the calculation of the "new" vehicle trips should be supported and well documented.

Sensitivity analysis should be undertaken where trip generation parameters have the potential to vary considerably and most probable values cannot be readily identified (i.e. the exact tenant or use of commercial space is not known). A trip generation table should be provided in the report to summarize the land uses and sizes, with the corresponding trip generation rates and the resulting number of trips. For large developments that will be phased in over time, the table should identify each significant phase separately.

7. Site Traffic - Trip Distribution & Assignment: Trip distribution assumptions should be supported by an adequate assessment of surrounding development, proximity to major trip generators, likely origin/destinations and existing/anticipated travel patterns. Sound engineering judgement should be utilized when establishing trip distribution.

Trip assignments should consider logical routings, available and projected road capacities, and travel times. Trip assignments may be estimated using a transportation demand model or "hand assignment" based on knowledge of the proposed/future road network in the study area. The trip assignment should also take into consideration varying assignments for pass-by trips, link-diverted trips and internal trips.

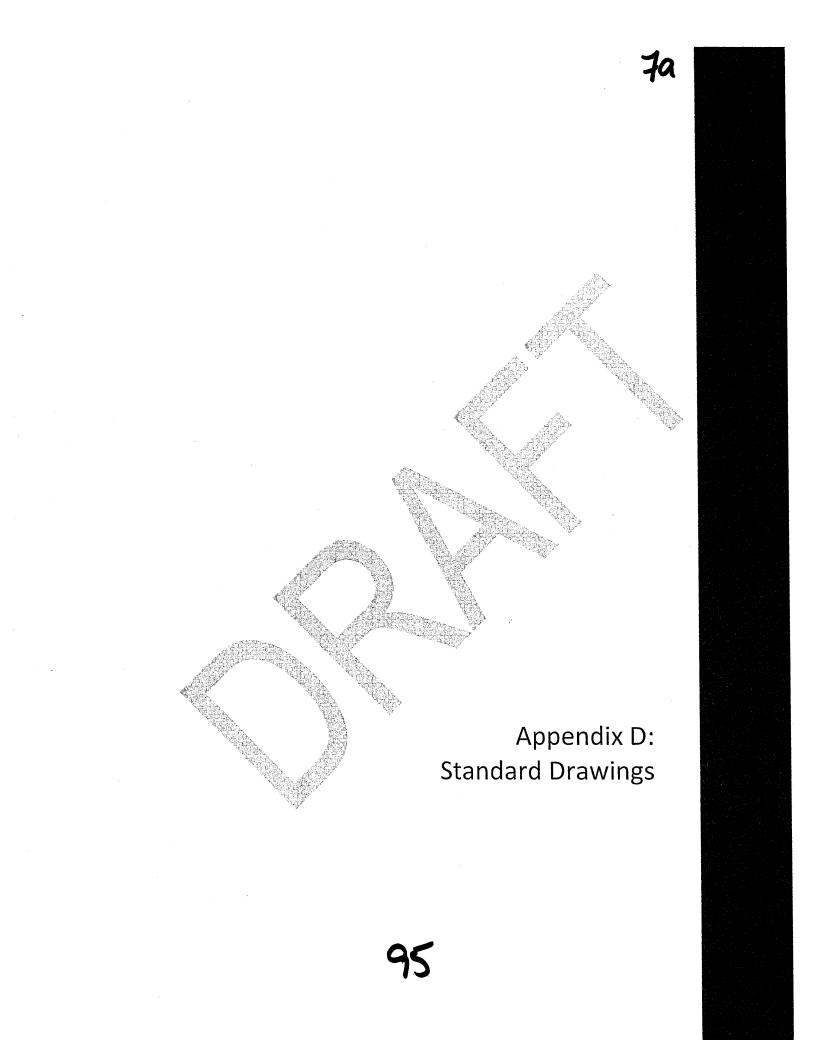
- 8. Future Total Traffic Volumes: Future total traffic volumes are to be presented, representing the culmination of the future background traffic volumes and the site generated traffic volumes for each horizon year.
- 9. Traffic Impacts: An evaluation of the key signalized and unsignalized intersections within the study area for all relevant peak periods for the existing and future planning horizons (both with and without the subject development traffic) is required and summaries are to be provided in a tabular format (e.g. level of service, volume to capacity ratio and delay). The intersection capacity/operational analysis should be completed using Synchro/Sim Traffic Simulation Software or the like. The objective should be to ensure that no new problem movements are created by the proposed development and that existing problem movements are not worsened to an unacceptable level with the addition of site generated traffic. Where intersections are closely located, queueing assessments should be included with respect to appropriate storage, queue encroachment, back to back left turns, etc.

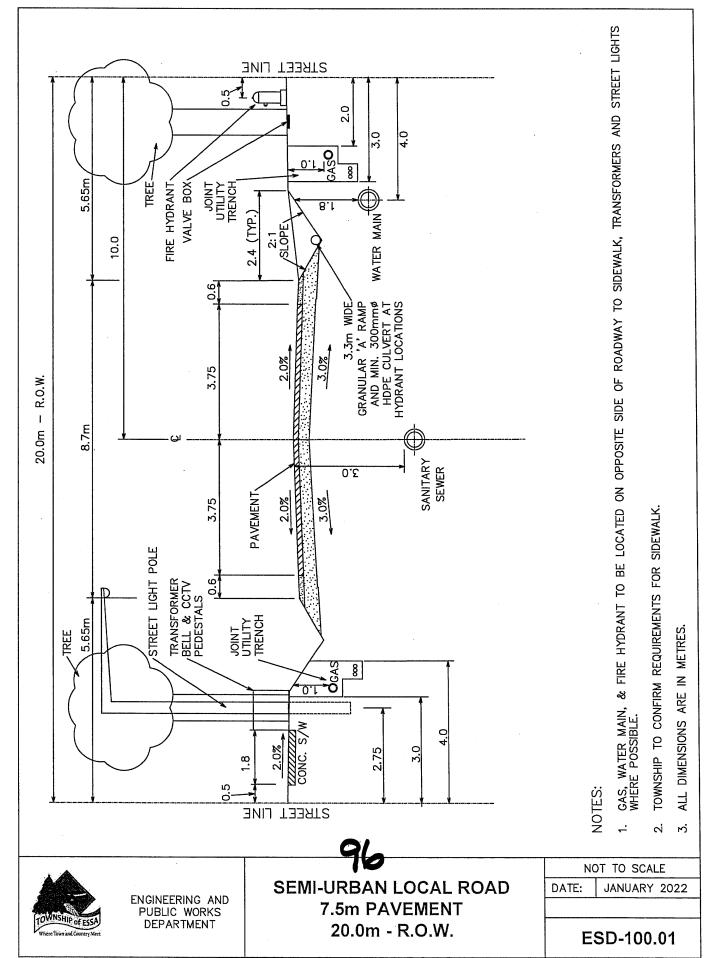
Should operational issues result, means of mitigation and the operations with consideration for such, are to be presented (e.g. introduction of traffic signals, additional through lanes, turn lanes, etc.) Appropriate warrants for improvements and associated operational review worksheets should be appended to the report.

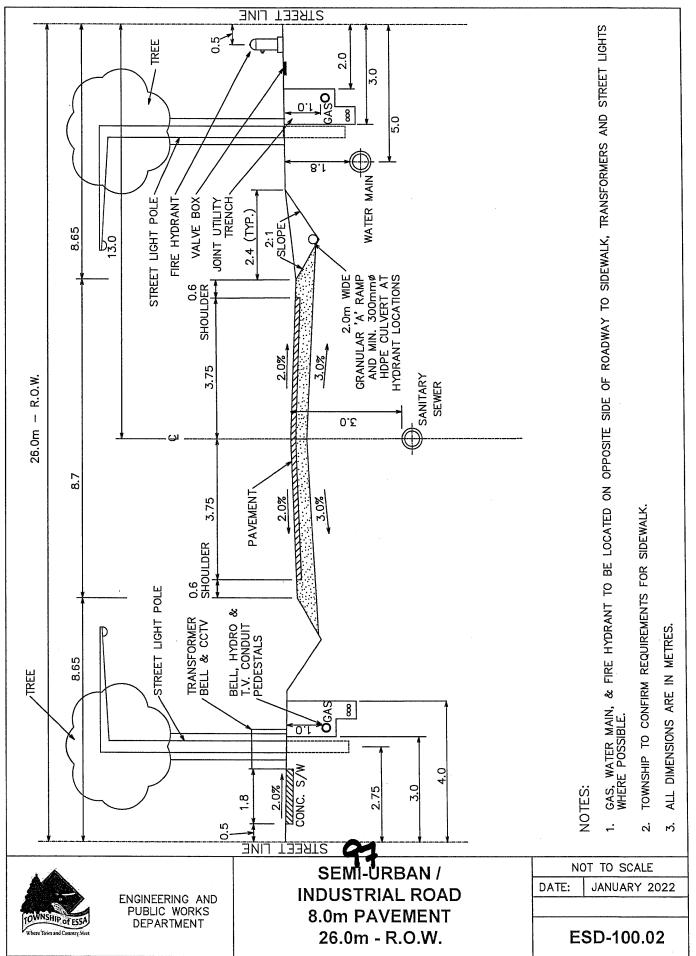
10. Site Access Assessment: The site access should be reviewed in consideration of the volume and type of vehicles to be generated by the site, and the proposed access location and configuration. The site access assessment should consider the Township's standards as outlined in *Township of Essa Engineering Design Standards and Specifications Manual*. Access points should be evaluated in terms of capacity, safety and adequacy of queue storage capacity.

Should operational issues result at the site access, means of mitigation and the operations with consideration for such, are to be presented (e.g. introduction of traffic signals, additional through lanes, turn lanes, etc.). Appropriate warrants for improvements and associated operational review worksheets should be appended to the report.

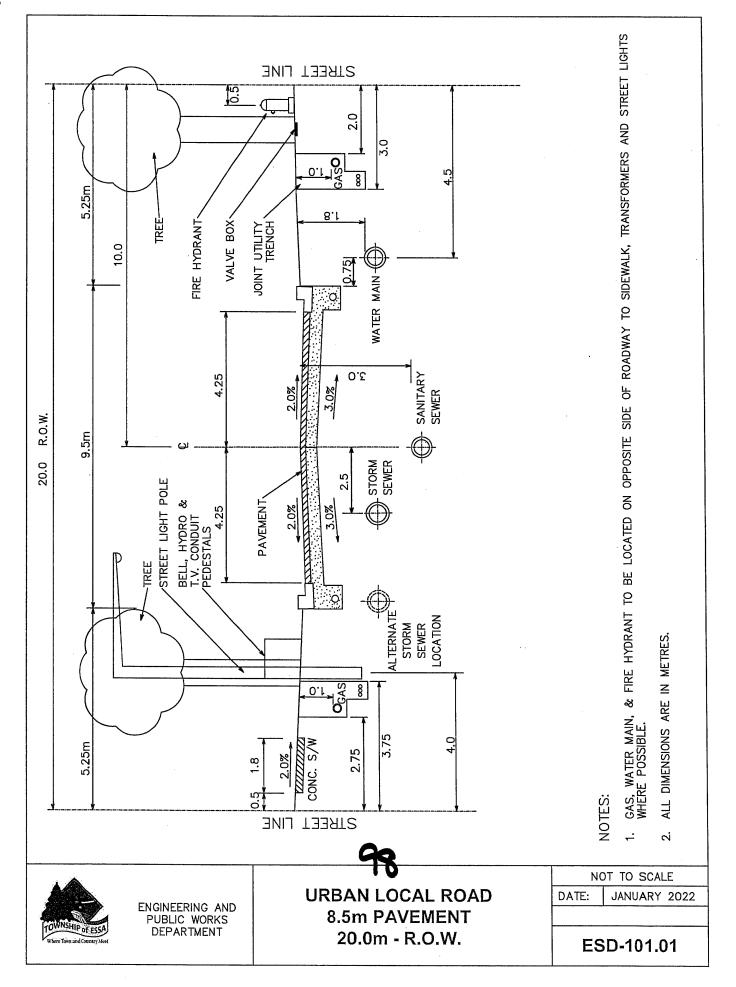
- 11. Sight Line Assessment: At each access point, the sight distance requirements should be determined based on the Township's guidelines (as detailed in *By-Law No. 2015-11* regulating entrances to Township roads), and the availability of appropriate sight lines determined from actual field measurements or engineering plans.
- 12. Site Plan Review: On-site circulation should be reviewed in context of the volume and type of vehicles to be served (including emergency vehicles, garbage collection or other appropriate design vehicles) and the ability of the site to accommodate site traffic without impact to the adjacent road system. Circulation of pedestrians and cyclists should also be considered as well the potential impact the development may have on existing pedestrian/cycling facilities. Queuing assessments may be required for land uses that induce on-site queues (i.e. restaurants with drive thru facilities, gas stations, drive-thru bank machines, car washes, etc.) The intent of a queueing assessment is to ensure that on-site queues can be accommodated wholly within the site without causing disruption to the operations of the adjacent road system (i.e. queue lengths that block access points or spill back onto the adjacent road.
- 13. Documentation: The structure and format of the Traffic Impact Study should follow the guidelines outlined in this document as applicable. This format will facilitate review, discussion and communication. Relevant maps, graphs, tables and figures should be provided.

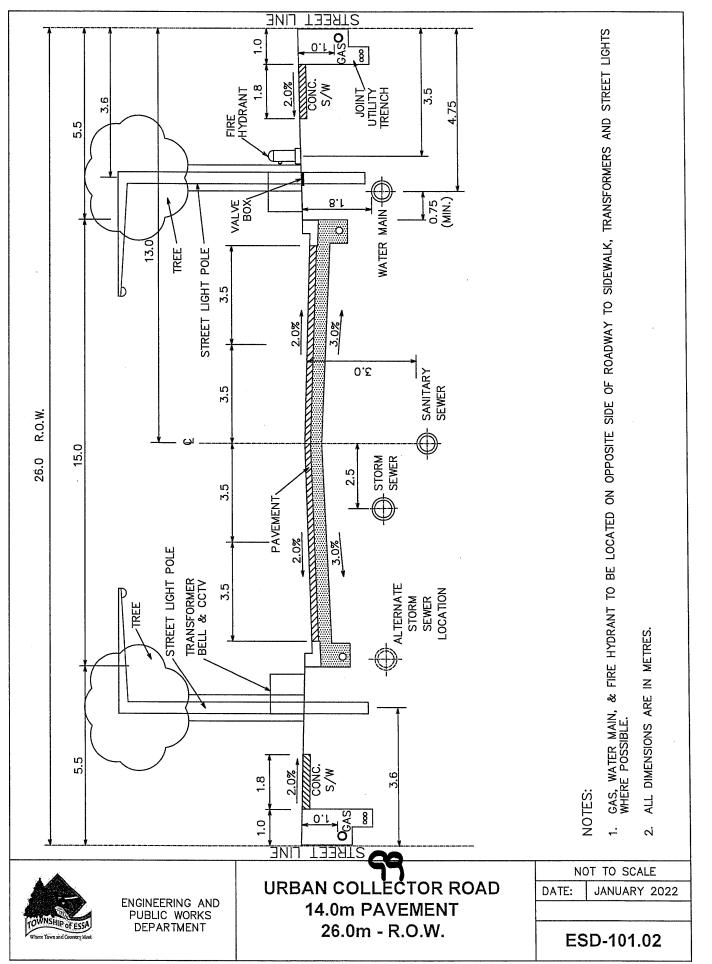




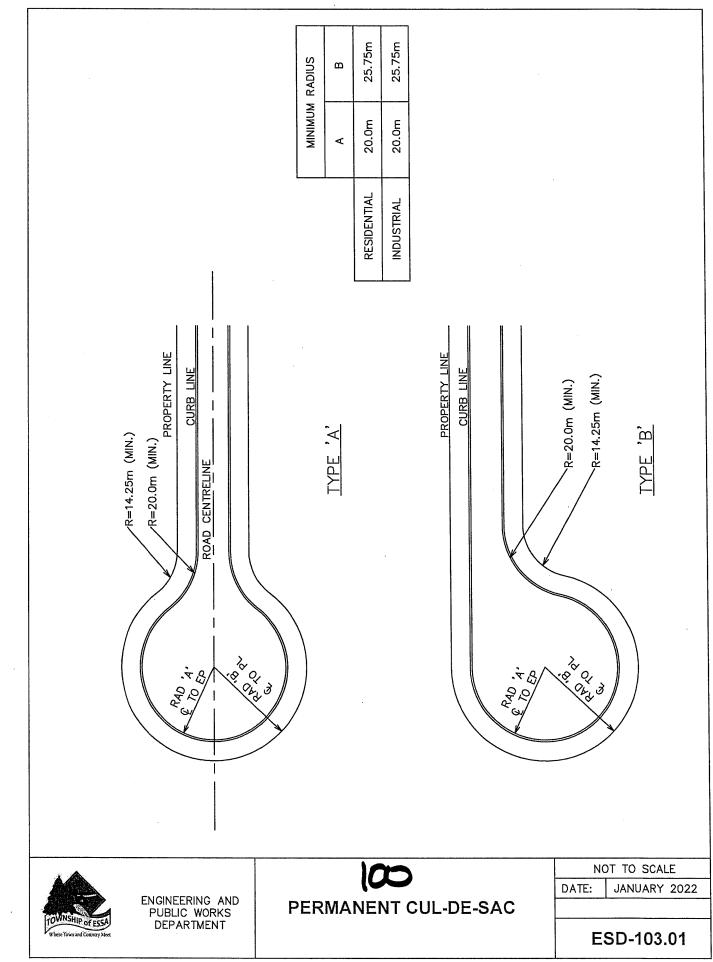


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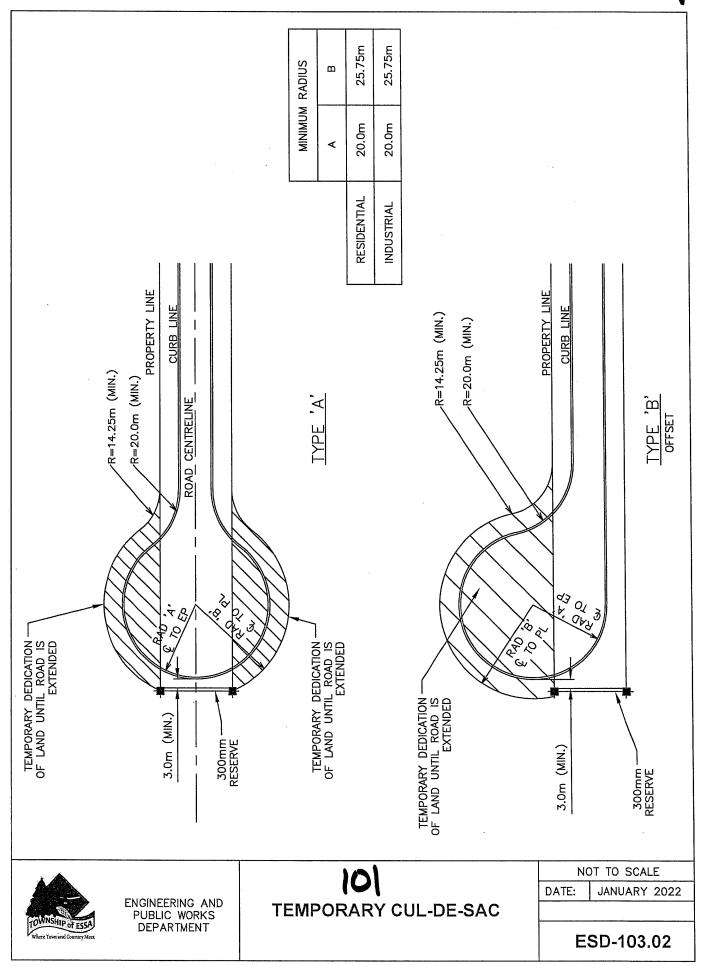




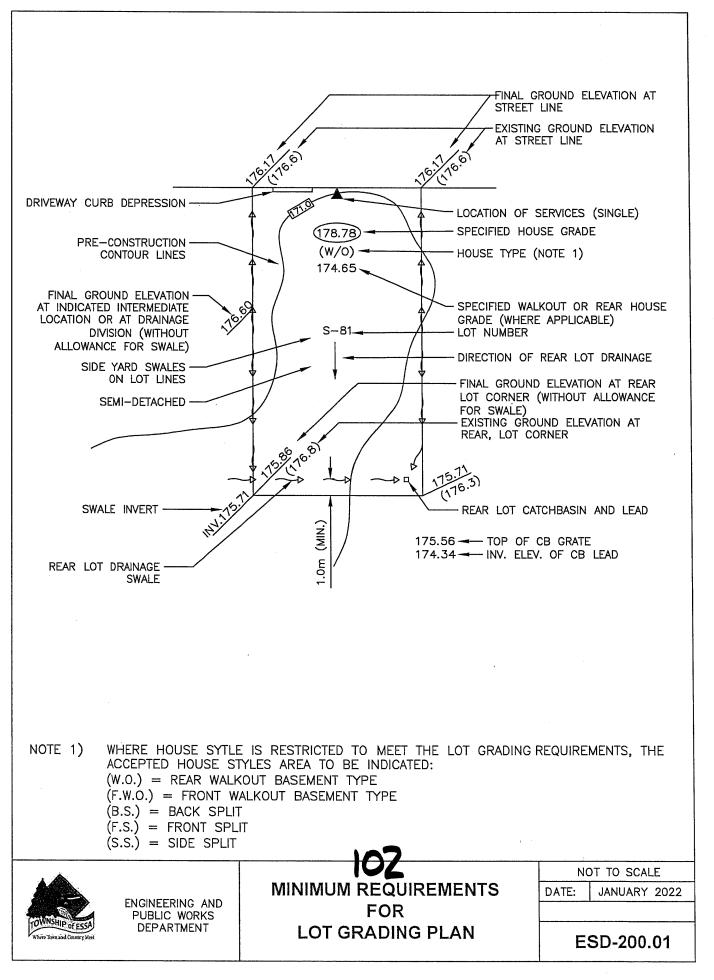
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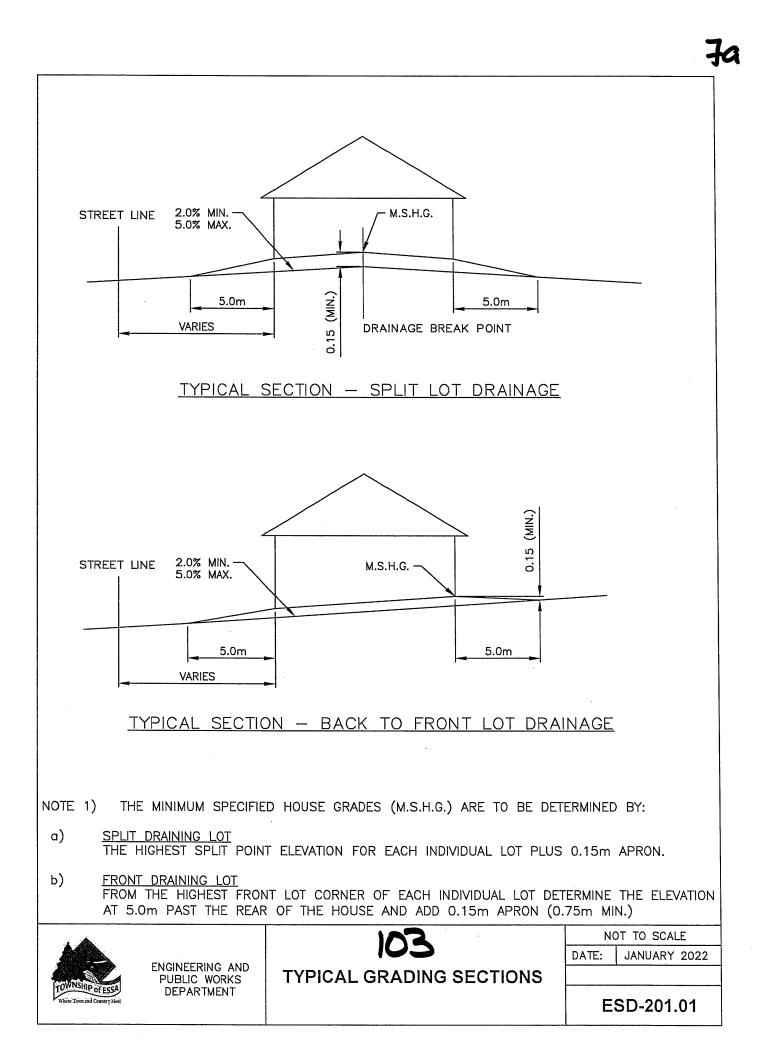


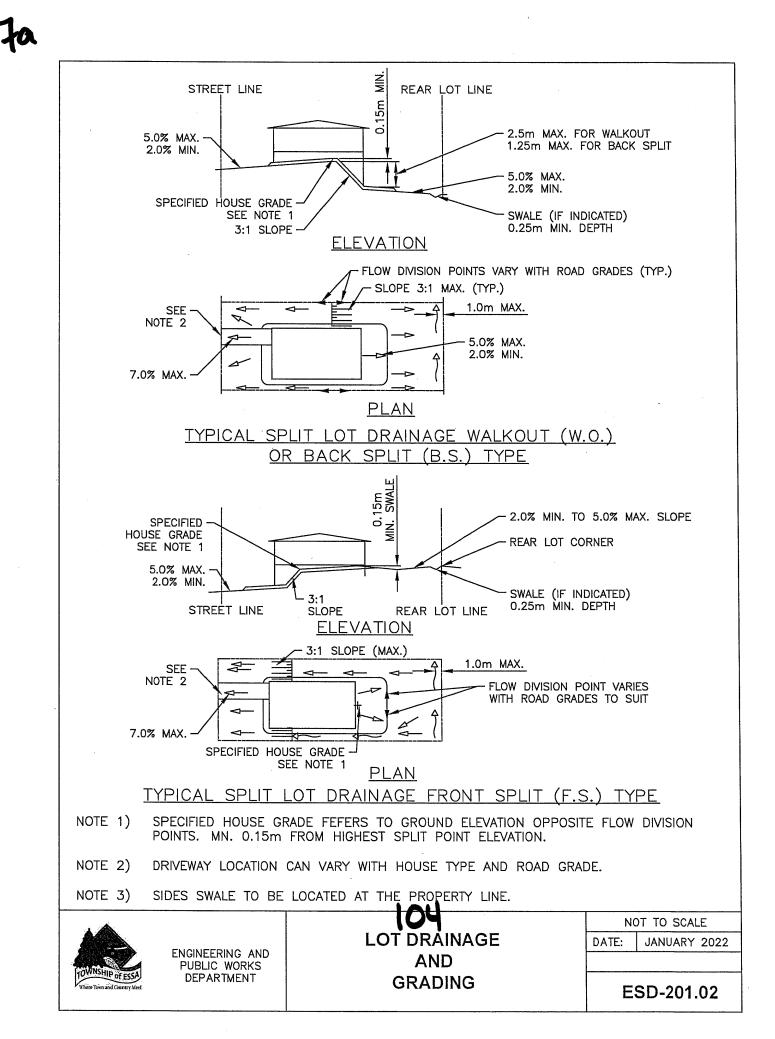
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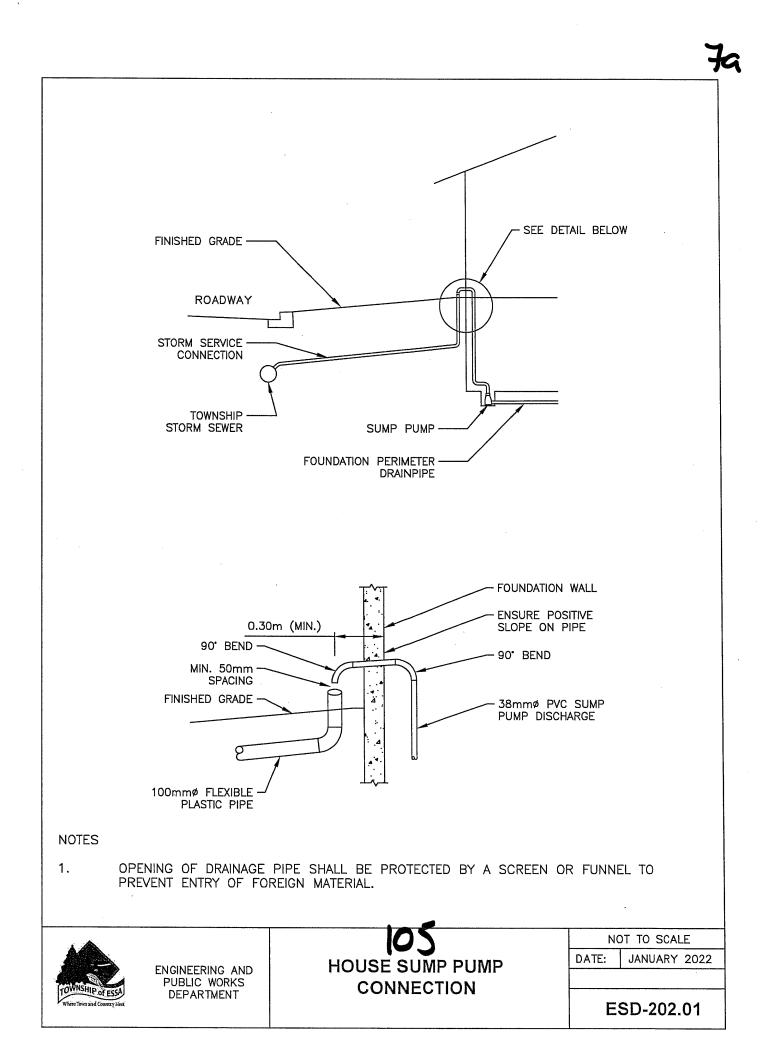


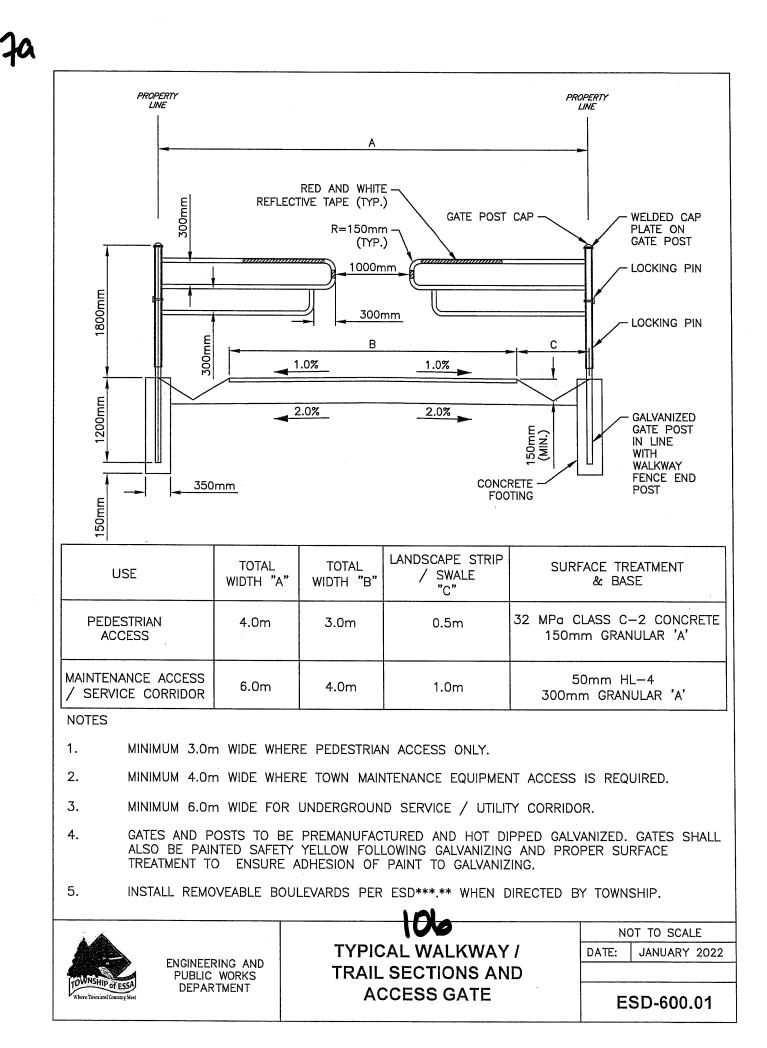
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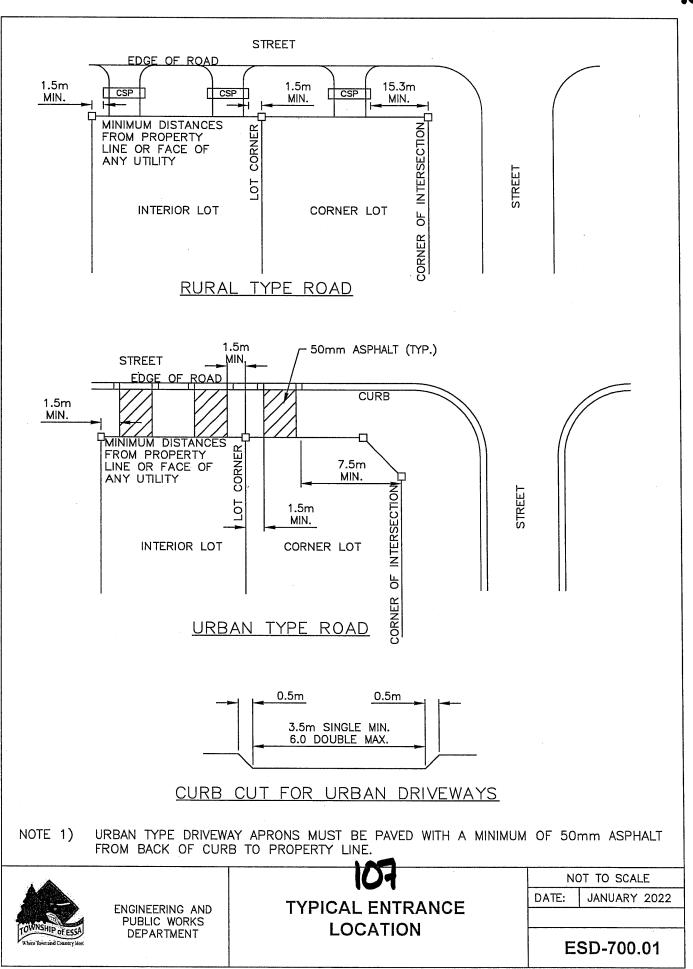




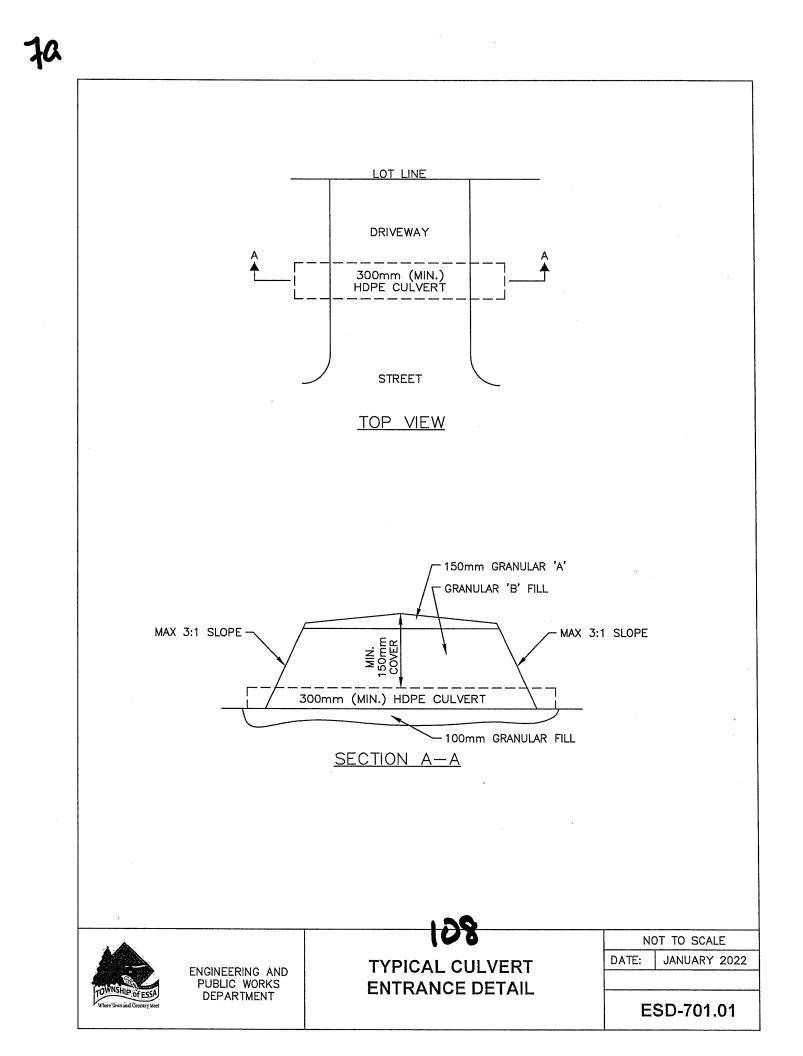


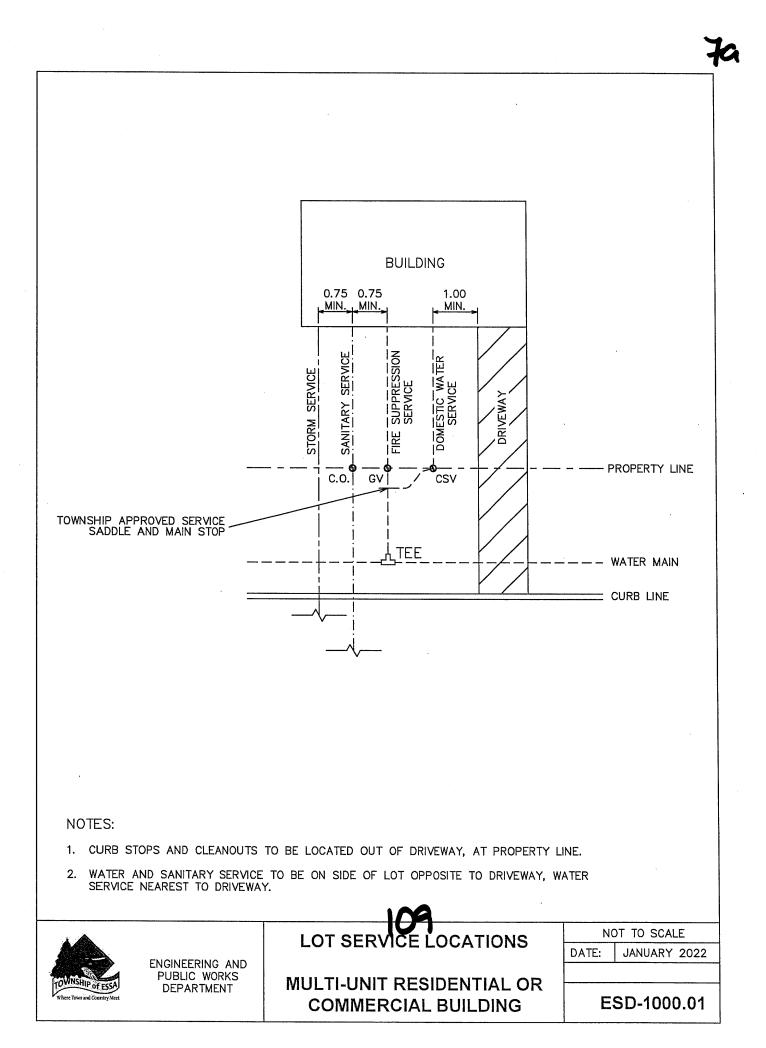


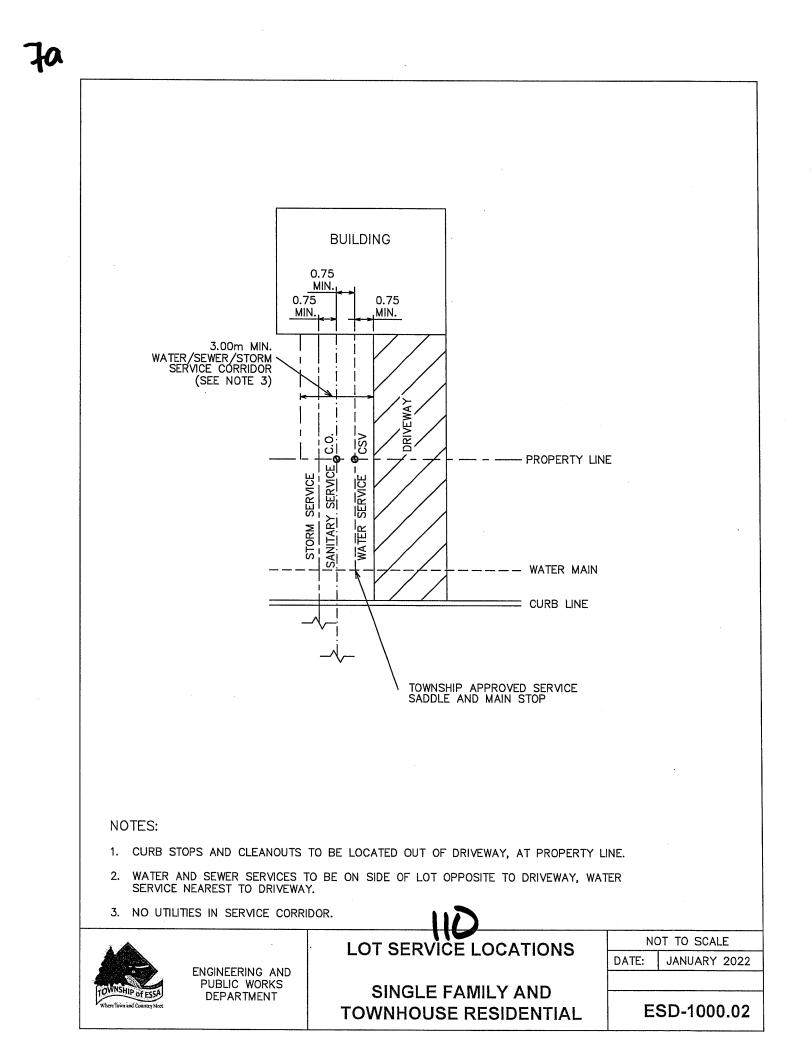


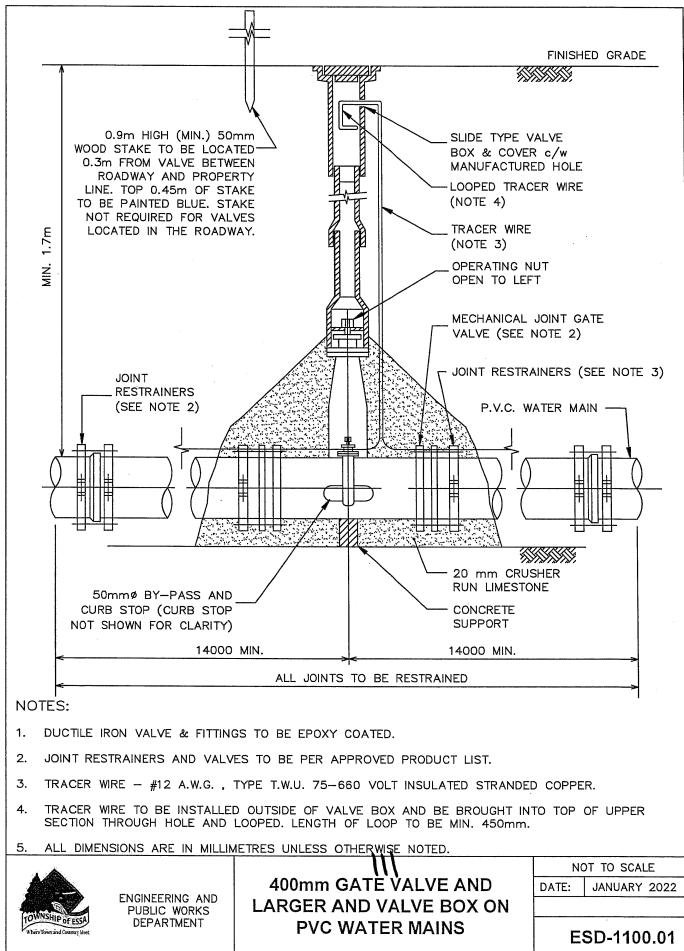


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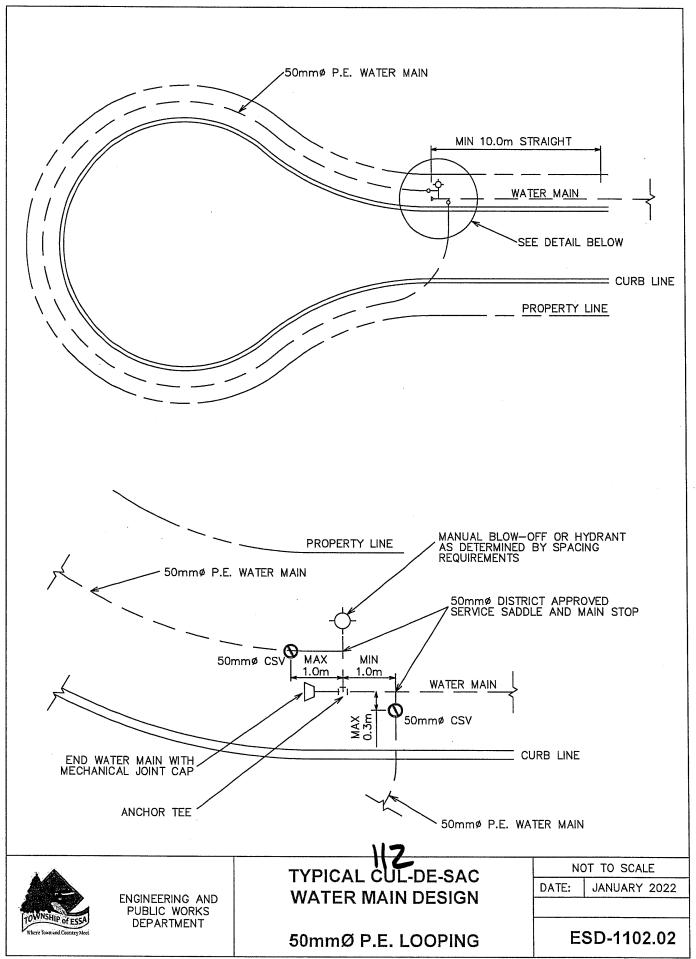












MINIMUM RESTRAINING LENGTH ON EACH SIDE OF FITTING PVC DR 18 WATERMAIN

BENDS

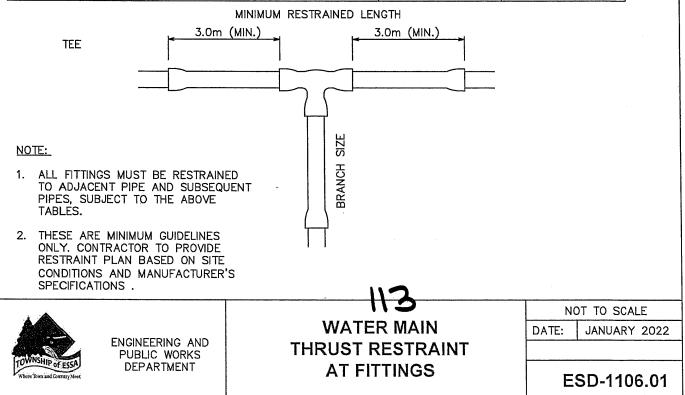
| HORIZONTAL BENDS, DEAD ENDS AND INLINE VALVES | 150mm ø PIPE SIZE (m) | 200mm ø PIPE SIZE (m) | 250mm ø PIPE SIZE (m) | 300mm ø PIPE SIZE (m) |
|--|--------------------------|--------------------------|--------------------------|--------------------------|
| 11.25 [°] | 0.6 | 0.6 | 0.6 | 0.9 |
| 22.5° | 0.9 | 1.2 | 1.5 | 1.5 |
| 45° | 1.80 | 2.4 | 2.7 | 3.4 |
| 90° | 4.27 | 5.5 | 6.7 | 7.9 |
| VALVES OR END CAPS | 9.1 | 12.2 | 14.4 | 17.0 |

REDUCERS

| | 150mm ø PIPE | 200mm Ø PIPE | 250mm ø PIPE | 300mm ø PIPE |
|--------------------------|-----------------|-----------------|-----------------|-----------------|
| | SIZE (m) | SIZE (m) | SIZE (m) | SIZE (m) |
| ONE PIPE SIZE SMALLER | 6.7 (100x150mm) | 6.4 (150x200mm) | 5.7 (200x250mm) | 5.7 (250x300mm) |

TEES (RUN SIZE)

| BRANCH SIZE | 150mm ø PIPE SIZE (m) | 200mm Ø PIPE SIZE (m) | 250mm ø PIPE SIZE (m) | 300mm ø PIPE SIZE (m) |
|-------------|--------------------------|--------------------------|--------------------------|--------------------------|
| 150mm | 0.3 | 0.3 | 0.3 | 0.3 |
| 200mm | 3.6 | 0.9 | 0.3 | 0.3 |
| 250mm | 7.6 | 5.8 | 3.4 | 1.2 |
| 300mm | 11.5 | 9.8 | 7.9 | 6.1 |





Township of Essa Marshall Subdivision (Brookfield) Review of External Works Securities

VIA EMAIL

June 21, 2022

File No. 217143

Township of Essa 5786 Simcoe County Rd. 21, Utopia, ON LOM 1T0

Attn: Michael Mikael, P. Eng Manager of Public Works

Re: Marshall Subdivision (Brookfield) Review of Development Securities – External Works Agreement

Dear Michael:

Further to the recent request for a security reduction received from the Developer's Engineer for the Marshall (Brookfield) Subdivision, we have reviewed the information and provide this recommendation for an adjustment in the development securities currently held by the Township.

As noted in the attached tables, securities in the amount of \$1,979,893.58 are still required to be retained by the Township to cover costs for: remaining incomplete and/or deficient works; 10% Engineering and Contingencies on those incomplete works; a maintenance security in the amount of 10% of the original cost of works; and a security holdback in the amount of 15% of all completed works, as noted in the External Works Agreement.

Included in the value of incomplete and/or deficient works within External Stage 2, Part S, is a holdback of 50% of Items 9 to 12 to address the grading/ditch slope concern on the west side of Denney Drive, from Murphy Road to Falhazi Trail. This value may be released upon construction of a solution to the satisfaction of the Township.

It is our understanding that the Township currently holds \$4,061,939.86 in securities for this development, which is more than the calculated required amount. Therefore, we recommend that the securities for this development be reduced from the current \$4,061,939.86 to \$1,979,893.59 for a total security reduction of \$2,082,046.27, to be returned to the Developer, provided that the Developer is in good financial standing with the Township.

We have attached the request for security adjustment received from the Developer's Engineer, for your reference.

We trust that this is satisfactory; however, if you have any questions or would like to discuss further, please do not hesitate to contact the undersigned.



Township of Essa Marshall Subdivision (Brookfield) Review of External Works Securities

Yours truly,

AINLEY & ASSOCIATES LIMITED

Thali-

T. Kalimootoo, P. Eng., PMP Vice-President & Branch Manager

\lag-barrie\ns1\Engineering\Barrie\217143\Contract Administration\Security Reductions\217143 - Development Security Review - 2022-06-21.docx

Encl.

cc: John Vanos – SCS Engineering Edward Kallikorm – Brookfield Development Rayna Thompson – Brookfield Development Nicole Kingston – Ainley Group



Township of Essa Marshall Subdivision (Brookfield) Review of External Works Securities

Security Summary Tables:

| PHASE OF DEVELOPMENT | VALUE OF INCOMPLETE WORK | | |
|--|--------------------------|--|--|
| External Works Agreeme | nt - Stage 1 | | |
| Site Servicing, Roadworks, Denny Drive (Fronting | Site), And SWM Pond | | |
| A. Mobilization and Site Preparation | \$38,000.00 | | |
| B. Sanitary Sewers | \$0 | | |
| C. Storm Sewers | \$27,695.00 | | |
| D. Clean Water Collector Sewers | \$0 | | |
| E. Watermain and Appurtenances | \$0 | | |
| F. Service Connections | \$960.00 | | |
| G. Stormwater Management Facilities | \$167,011.09 | | |
| H. Denney Drive External Site Servicing and Roadworks | \$30,905.00 | | |
| I. Gauley Drive | \$58,375.50 | | |
| SWM Pond Outfall | | | |
| J. Mobilization and Site Preparation | \$0 | | |
| K. Removals | \$0 | | |
| L. Storm Sewers | \$0 | | |
| M. Roadworks | \$26,888.99 | | |
| Landscaping – SWM Pond | | | |
| N. SWM Pond | \$245,210.00 | | |
| Streetlighting (Denney Drive) And Intersection Pedestrian Signalization | | | |
| O. Streetlighting (Gauley Drive) | \$23,000.00 | | |
| P. Streetlighting (Externals – Denney Drive) | \$63,146.00 | | |
| Q. Intersection Pedestrian Signalization (Denn Drive & Felhazi Trail) | ey \$43,408.00 | | |
| R. Intersection Pedestrian Signalization (Denn Drive & Murphy Road) | ey \$43,859.00 | | |
| Sub-Total Externals Stage | \$1 \$768,458.58 | | |



| PHASE OF DEVELOPMENT | VALUE OF INCOMPLETE WORK | |
|---|--------------------------|--|
| External Works Agreement | - Stage 2 | |
| Site Servicing, Roadworks, And Denney Drive | | |
| S. Denney Drive External Site Servicing and Roadworks | \$145,457.15 | |
| Landscaping | | |
| T. Denney Drive | \$5,100.00 | |
| Sub-Total Externals Stage 2 | \$150,557.15 | |

| DESCRIPTION | REQUIRED SECURITIES |
|--|---------------------|
| Total Value of Incomplete Work, Stage 1 & Stage 2 (From Above) | \$919,015.73 |
| 10% Engineering and Contingencies on Incomplete Work | \$91,901.57 |
| 10% Maintenance Holdback (on original cost of all works - \$4,482,455.52) | \$448,245.55 |
| 15% Security Holdback (on value of all completed works - \$3,471,538.21) | \$520,730.73 |
| TOTAL SECURITIES REQUIRED | \$1,979,893.58 |

Spring 2022

Integrity Commissioner's Annual Report Township of Essa

Principles *Integrity* is pleased to submit this annual report, covering the period from February 1, 2021, the date of our last annual report, and May 31, 2022.

The purpose of an Integrity Commissioner's annual report is to provide the public with the opportunity to understand the ethical well-being of the Township elected and appointed officials through the lens of our activities.

About Us:

Principles *Integrity* is a partnership focused on accountability and governance matters for municipalities as well as other locally-elected and appointed governing bodies. Since its formation, Principles *Integrity* has been appointed as Integrity Commissioner (and as Lobbyist Registrar/Closed Meeting Investigator/Municipal Ombudsman for some clients) in over 50 Ontario municipalities and other public bodies. Part of our mission is to advance the Municipal Integrity Commissioner of Ontario (MICO), where we have led development of MICO's response to the Provincial consultations and Ombudsman Office relations.

The Role of Integrity Commissioner, Generally:

An Integrity Commissioner's statutory role is to carry out, in an independent manner, the following functions:

- Advice on ethical policy development
- Education on matters relating to ethical behaviour
- Providing on request, advice and opinions to Council, members of Council and members of Local Boards
- Providing a mechanism to receive inquiries (often referred to as 'complaints') which allege a breach of ethical responsibilities
- Resolving complaints informally, where appropriate, and
- Investigating, reporting and making recommendations to Council on those complaints that cannot be resolved informally, while being guided by Council's codes, policies and protocols.

This might contrast with the popular yet incorrect view that the role of the Integrity Commissioner is primarily to hold elected officials to account; to investigate alleged transgressions and to recommend 'punishment'. The better view is that Integrity Commissioners serve as an independent resource, coach, and guide, focused on enhancing

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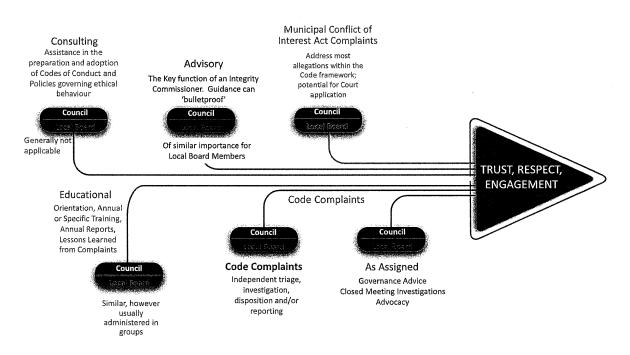
the municipality's ethical culture.

The operating philosophy of Principles *Integrity* recites this perspective. We believe there is one overarching objective for a municipality in appointing an Integrity Commissioner, and that is to raise the public's perception that its elected and appointed officials conduct themselves with integrity:

The perception that a community's elected representatives are operating with integrity is the glue which sustains local democracy. We live in a time when citizens are skeptical of their elected representatives at all levels. The overarching objective in appointing an Integrity Commissioner is to ensure the existence of robust and effective policies, procedures, and mechanisms that enhance the citizen's perception that their Council (and local boards) meet established ethical standards and where they do not, there exists a review mechanism that serves the public interest.

The practical effect of achieving this objective is an increase in trust, respect and engagement in local affairs.

In carrying out our broad functions, the role falls into two principal areas. 'Municipal Act' functions, focused on codes of conduct and other policies relating to ethical behaviour, and 'MCIA' or *Municipal Conflict of Interest Act* functions. From an activity perspective, an Integrity Commissioner's role can be depicted this way:



The emphasis of Principles *Integrity* is to help municipalities enhance their ethical foundations and reputations through the drafting of effective codes of conduct and other policies

governing ethical behaviour, to provide meaningful education related to such policies, and to provide pragmatic binding advice to Members seeking clarification on ethical issues. As noted in the graphic, we believe that the support we give to Members of Council increases the public's perception of them, which in turn leads to greater trust, respect and engagement.

Because the development of policy and the provision of education and advice is not in every case a full solution, the broad role of the Integrity Commissioner includes the function of seeking and facilitating resolutions when allegations of ethical transgressions are made, and, where it is appropriate and in the public interest to do so, conducting and reporting on formal investigations. This in our view is best seen as a residual and not primary role.

Confidentiality:

Much of the work of an Integrity Commissioner is done under a cloak of confidentiality. While in most cases secrecy is required by statute, the promise of confidentiality also encourages full disclosure by the people who engage with us. We maintain the discretion to release confidential information when it is necessary to do so for the purposes of a public report, but those disclosures would be limited and rare.

Our Activity for the Township:

As municipalities head into an election year, we reflect back on the past term of Council from our vantage point as Integrity Commissioner. Members of Council, individually and collectively, have demonstrated a solid understanding and commitment to ethical conduct and behaviour. Where concerns have arisen, we have been pleased to provide advice and guidance to support Members in meeting the high standards expected by the public, and in assisting the public in holding their elected representatives accountable for the ethical conduct.

During the period covered by this report, we have been engaged in a moderate level of activity as Integrity Commissioner for the Township which subdivides roughly into three categories:

1. Policy Development and Education

During the period covered by this report, there has not been the opportunity for us to be engaged in any policy development or education with the municipality.

2. Advice

The advice function of the Integrity Commissioner is available to all Members of Council and where applicable their staff and Members of local boards on matters relating to the code of conduct, the *Municipal Conflict of Interest Act* and any other matter touching upon the ethical conduct of Members. Advice provided by the Integrity Commissioner is confidential and independent, and where all the relevant facts are disclosed, is binding upon the Integrity Commissioner.

Our advice is typically provided in a short Advice Memorandum which confirms all relevant facts and provides with clarity our analysis and a recommended course of action.



During the period covered by this report, we responded to 4 such requests for advice.

3. Complaint Investigation and Resolution

Our approach to reviewing complaints starts with a determination as to whether an inquiry to us is within our jurisdiction, is beyond a trifling matter, is not either frivolous or vexatious, and importantly, whether in its totality it is in the public interest to pursue. We always look to the possibility of informal resolution in favour of formal investigation and reporting. Once a formal investigation is commenced, the opportunity to seek informal resolution is not abandoned.

Where we are able to resolve a matter without concluding a formal investigation, our practice is to provide a written explanation in the form of a Disposition Letter to the complainant to close the matter. Often the respondent Member is involved in preliminary fact-finding and will also be provided with a summary of the disposition.

Where formal investigations commence, they are conducted under the tenets of procedural fairness and Members are confidentially provided with the name of the Complainant and such information as is necessary to enable them to respond to the allegations raised.

During the period covered by this report we responded to one complaint which did not result in a full investigation and report to Council.

Ethical Themes Around the Province:

With due regard to our obligation to maintain confidentiality, this annual report enables us to identify learning opportunities from advice requests and investigations conducted in a variety of municipalities.

Disclosure of confidential information from closed meeting sessions

There have been some examples where elected or appointed officials fail to recognize the serious implications of disclosing confidential information, particularly information learned of through attendance in closed session.

A Member's obligation to maintain confidentiality is clear. They may not unilaterally decide to share confidential information, even if they believe the information should be publicly disclosed. This extends to releasing information even to their own legal counsel to obtain a 'second opinion'.

We treat this breach of ethical responsibility as breach of a cardinal rule, and if an allegation in this regard is proved to be true, it tends to attract a recommended sanction at the upper end of the prescribed range. Left unchecked, confidentiality breaches to undermines not only Council's interests on the matter subject to the breach, but destroys the trust required of elected officials, and the staff that support them, to ensure that all relevant, and sensitive, information required to support the deliberation on a matter is freely supplied.

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Non-disparagement

One area of prominence continues to be the failure of some Members of Council to adhere to rules against disparagement. Members of Council are entitled, and indeed expected to disagree on all manner of issues. However, one of the cornerstones to democracy must be the recognition that different opinions and perspectives are to be respected, and disagreement should not devolve into disrespect, disparagement and name-calling.

Disrespectful interactions and/treatment of others can fall along a continuum which may manifest as occasional incivility and micro-aggressions, but when unchecked can culminate in bullying and harassment. Members of Council should be mindful to treat each other, staff and the public with appropriate respect and professionalism at all times.

Some Members of Council hold a view was that they are entitled to their freely express their opinion, even if that includes disparagement of others, and so long as they share it via personal email, and not on the municipal server, they are not constrained by any rules around decorum. This is incorrect. Members are bound by the Code provisions of respectful and non-disparaging communication, whether sharing views on their own email, social media, or elsewhere.

Regardless of the medium, regardless of the intended audience, and regardless of motive, we have observed several instances where Members of Council in municipalities around the province have been found to have breached ethical standards by saying or recording things they have come to regret.

Recognizing and avoiding conflicts of interest

Another area Members frequently require additional clarification on is recognizing and appropriately identifying conflicts of interest when they arise. These often include when members are part of another organization or club whose interests are impacted by a matter before Council, or when members are active professionally within the community and a matter before Council may potentially impact one of their current or past clients.

Take-aways from the Collingwood Inquiry

The Collingwood judicial inquiry published its report, entitled "Transparency and the Public Trust: Report of the Collingwood Judicial Inquiry," on November 2, 2020. This Report contained 306 recommendations relating to best practices in municipal governance.

In particular, the Report contained important discussion regarding the rules relating to conflicts of interest applied to municipal councillors under the Municipal Conflict of Interest Act ("MCIA"), which are overly narrow, focusing on pecuniary (i.e. monetary) interests. The Report warns against considering "pecuniary interest" as the sole criterion in assessing whether a councillor is subject to a conflict of interest:

...it is far too easy to misconstrue the Municipal Conflict of Interest Act as addressing all the kinds of conflict of interest that Council members must



confront. Despite its name, the Municipal Conflict of Interest Act does not provide a complete conflict of interest code for municipal actors. It addresses the pecuniary interests of a narrowly defined group of family members related to a Council member which are by virtue of the Act deemed to be pecuniary interests of the Council member. Council members are obligated to avoid all forms of conflicts of interest or, where that is not possible, to appropriately disclose and otherwise address those conflicts.

As always, obtaining clear and reliable advice from the Integrity Commissioner can help avoid costly and time-consuming investigations if there is any uncertainty about the application of the Rule.

Staying in your lane

One area of concern that continues to arise is members of Council overstepping their role, attempting to 'take the reins' to fix a constituent's problem, or directing staff how to do their job. Members of Council serve an important role in putting constituents in touch with appropriate staff, and following established processes, but it is important to strike the correct balance.

At times inappropriate interference arises because of a misinterpretation of the *Municipal Act* provisions which identify the role of the Head of Council as 'Chief Executive Officer'. This provision has led to confusion and, occasionally, overreach by Heads of Council in erroneously perceiving a role leading the municipality's administration. The Province has been requested by MICO, the Ombudsman and others, to amend the *Municipal Act*, to clarify that elected officials – even Heads of Council – have no role in the day-to-day administration of municipal government.

Failing to recognize this, stepping outside of their proper role as elected officials to 'take the reins' of administration, undermines staff and can be perceived as interfering with management. This overstepping of the proper role by Members, even Mayors, must be recognized as inappropriate under the Code of Conduct and the Council-Staff Relations Policy, both mandated under the *Municipal Act*.

As always, obtaining clear and reliable advice can help avoid a costly and time-consuming investigation.

Conclusion:

We look forward to continuing to work with Members of Council to ensure a strong ethical framework. We embrace the opportunity to elevate Members' familiarity with their obligations under the Code and to respond to emerging issues. As always, we welcome Members' questions and look forward to continuing to serve as your Integrity Commissioner.

It has been a privilege to assist you in your work by providing advice about the Code of Conduct and resolving complaints. We recognize that public service is not easy and the



ethical issues that arise can be challenging. The public rightly demands the highest standard from those who serve them, and we congratulate Council for its aspirational objective to strive to meet that standard.

Finally, we wish to thank the Clerk and the Chief Administrative Officer for their professionalism and assistance where required. Although an Integrity Commissioner is not part of the administrative hierarchy, the work of our office depends on the facilitation of access to information and policy in order to carry out the mandate. This was done willingly and efficiently by the staff.

All of which is respectfully submitted -

Principles Integrity Integrity Commissioner

Jeffrey A. Abrams and Janice Atwood-Petkovski, Co-Principals postoffice@principlesintegrity.org



TOWNSHIP OF ESSA STAFF REPORT

| STAFF REPORT NO.: | CAO22-22 |
|-------------------|---|
| DATE: | June 29, 2022 |
| TO: | Committee of the Whole |
| FROM: | Colleen Healey-Dowdall, Chief Administrative Officer |
| SUBJECT: | 152 Greenwood Drive Rezoning of Land to Allow for a School |

RECOMMENDATION

That Staff Report CAO22-22 be received for information; and

That the CAO collect further information to use, to present to Council, to satisfy the comments and/or concerns of the public; and that a further report be presented to Council at a future date.

BACKGROUND

Council is considering to rezone 152 Greenwood Drive from Open Space Park (OSP) Zone to Institutional (I) Zone to allow for an added use of a school in addition to a park use. 152 Greenwood Drive is legally described as Block 11 on Plan 51M-784. It is a part of a plan of subdivision which was draft plan approved by the OMB in 1995 for the developer, Stonemount Developments. Neighbours at the time had opposed on the basis of poor traffic circulation and the municipality and developer disagreed over development charges.

When the OMB approved the 904-home subdivision in 1995, it imposed conditions to require different entry-roads or points of access to allow for good traffic movements and circulation and the construction of Willoughby Road and Bridge over the Nottawasaga River to provide for a new road leading in and around Angus. Good connectivity is still a principle of good planning today with roadways now also intended to be shared with those walking or wheeling. The original plan of subdivision contained 2 school sites with lands set out for both the public and catholic school boards.

Sometime later, in or about 2012, the developer approached both school boards and both agreed to relinquish the school blocks to the developer, although Essa Township opposed. The school boards employ their own Planners, and both rely on a Ministry of Education formula for the planning of schools (which is not as far reaching a formula as municipal official plan documents which traditionally consider a 20-year horizon).



More recently, approximately 5 years ago, the Simcoe County District School Board (SCDSB) reached out to municipal staff to make enquiry on lands available for another elementary school in Angus. This staff member, the municipal Manager of Planning & Development of the day, provided school board staff with mapping to show <u>all vacant lands in Angus</u>, i.e., vacant lands existing on Huron Street, Centre Street and on County Road 90. The SCDSB reached out to several landowners and carried out considerable inquiry and investigation on varying parcels. It was made clear to Essa staff that the SCDSB required a land holding of a certain size (2 hectares/5 acres) and they preferred the south end of Angus where most new growth is locating.

It should be noted that immediately adjacent to the Stonemount subdivision and abutting to the south lies another 350 homes in the Brownley Meadows subdivision, now built out, and immediately adjacent and abutting to the north lies another 100-home subdivision, now built out. There have been other further "buildouts" of considerable size nearby on Centre Street within 500 metres.

Good planning principles of the day remain based on the premise that land use patterns should promote a mix of uses that increase the use of active transportation (i.e., walking); strong and healthy communities promote human health and social well-being and promote a resiliency to climate change; and intensification and redevelopment in built-up areas should occur in order to promote an efficient means of using infrastructure (Provincial Policy Statement, 2020).

Planning in the province today is governed by the Provincial Policy Statement, 2020; and A Place to Grow: Growth Plan for the Greater Golden Horseshoe. Planning in Essa Township is guided by the County of Simcoe Official Plan and Essa Township's Official Plan (OP). The entire Stonemount subdivision is designated as Residential – Future in Essa's OP. This designation is broadly applied throughout Angus to show where subdivisions are going and the subdivision or resulting neighbourhood may contain any one of amenities associated with residential neighbourhoods including schools, parks or neighbourhood plazas. This designation refers to the approved plan of subdivision to be more specific as is implemented by the Zoning By-law.

COMMENTS AND CONSIDERATIONS

In the case at hand, the subject lands lie in Angus, Essa Township's primary urban centre, which contains the bulk of Essa's population and where most growth has occurred over the past couple of decades and will continue to be focussed. Essa is in the midst of preparing a new OP document to guide future land use in Essa Township. The expectation that Angus will continue to be Essa's primary growth centre is not likely to change with the foundation of community services and infrastructure investment already in place.

The lands lie within a primarily Residential area and a public elementary school is in keeping with the spirit and intent of the designation/zone wherein lands are set out for public purposes. The school, land and buildings, may be enjoyed by many in the neighbourhood since often, school yards are used for public park purposes after hours,

i.e., equipped with outdoor basketball nets, fields are used for soccer games by soccer organizations, and gymnasiums are rented for activities including club meetings/events.

The current proposal, a joint venture of the Township and SCDSB, is one whereby the Township would sell an existing park at 152 Greenwood Drive to the SCDSB and where the Township would build new parks elsewhere, <u>nearby</u>, with the intent that the public will not be without a community park. The current built portion of the parkland at 152 Greenwood Drive contains a parking lot, outdoor pad, splash pad and play structures and covers approximately 8,500 square metres of area.

The Township owns approximately the same amount of cumulative area of vacant land at the following distances from 152 Greenwood Drive, land area which is usable for park purposes.

| Location | Approx Area (sq. metres) | Approx Distance from 152 |
|-----------------------|--------------------------|--------------------------|
| | | Greenwood Drive (m) |
| 191 Maplewood Drive | 6,435 sm | 400 m |
| 60 Mike Hart Drive | 8,200 sm | 1,200 m |
| Gold Park Gate | 12,000 sm | 120 m |
| 84 Greenwood Drive | 700 sm | 50 m |
| 83 Greenwood Drive | 800 sm | 50 m |
| Total land available: | 28,135 sm | |

Refer to the mapping attached as Attachment 1.

The following was explained to the public at the statutory public meeting held on June 15, 2022:

..... the municipality has arranged for an agreement with the SCDSB wherein the Board will be purchasing 152 Greenwood Dr for use for a future elementary school if funding becomes available. Until such time that funding for a school becomes available, the municipality will continue to offer the public the use of the lands as a park. All park equipment will remain intact until such time as a new school is supported by the Ministry of Education. The municipality plans to use other public lands nearby as shown on the map below, for park purposes. All equipment will be replaced with new equipment to be under warranty. As such, the parks dept may begin planning for new parks with public input, to provide for parks with equipment requested by our citizens.

The SCDSB has listed Angus as a top priority for a new elementary school although no one knows what year that funding may be approved. Once Ministry funding is approved, it could be a couple of years before a school is able to be constructed since the site will have to be cleared and a plan and design put in place. The site will have to be planned with the school itself placed closer to the south limit of the property than the north given that



the north limit borders the County of Simcoe corridor set out for either rail or trail purposes. The County has asked that the Board respect a setback as if trains were using the rail line. A school will be designed to be in keeping with the neighborhood.

Once the site is able to be cleared, then siltation fencing will be erected along with construction fencing to minimize dust and to keep children safely off the construction site. It is hoped that the construction phase may only last 2 years.

The SCDSB is looking to secure other Angus locations for a fourth elementary school and the Simcoe Muskoka Catholic District School Board (SMCDSB) is also looking for land for an elementary school.

All funds received from the sale of parkland must, according to the Planning Act, be put towards park purposes. The municipality will be requiring the Board to design a site to accommodate for parent drop offs and pick ups and to include traffic calming in the site design. The municipality may wish to explore traffic calming on the street in connection with an overall traffic design study for the area.

There were 8 members of the public who spoke at the public meeting, 5 of these whom had emailed comments ahead of the public meeting. 3 others had emailed concerns in advance of the public meeting and there have been 4 new emails received after the public meeting but in advance of writing this report on June 20th. The concerns of the public can be summed up as follows:

- 1. Traffic
 - Residents anticipate a lot of traffic in association with a new elementary school
 - More congestion/volume
 - Speeding
 - Parking problems on streets
 - Pick up/drop off
 - Concern for safety for children/pedestrians
- 2. Environmental
 - Additional development can add to existing drainage problems/pavement can cause further/new drainage problems
 - Concern for floodplain/wetland/impact on aquifer on other lands
 - Cutting trees for the new park a concern for reasons listed above
 - Grading concerns due to topography
- 3. People bought <u>expecting</u> a park at 152 Greenwood Drive and their children, or children being "babysat", use the adjacent park
- 4. People <u>paid a premium</u> to buy onto vacant land nearby or beside the park/land value

Certainly, there were those who have acknowledged that a new school is needed (although many students will continue to require busing to a catholic school, french school and high schools) and that this opportunity could improve on walking and climate impact. As well, there was a comment to request new forms of parks with amenities geared to all ages.

Discussion of Comments and Concerns

This is a busy neighbourhood with approximately 1500 homes within a 1 km radius. Many people purchased expecting a park at 152 Greenwood Drive and prior to this, not that long ago, at least 1 school was expected/planned. The public will not lose a community park, albeit a park at 152 Greenwood Drive, and later, will surely be considered as 'better off' with more, and an improved range of amenities. The Township intends to construct new parks prior to the start of the new school. As well, a school site itself serves somewhat in the same function or role as a park with the school yard available after school hours and use of a gymnasium often for rent.

In fact, one school committee representative has suggested that it could be, potentially, a minimum of 5 years before a school at 152 Greenwood Drive and with the SCDSB providing permission through an agreement to allow for the continued use of the property as the existing park, the Township and residents should have the original park plus replacement(s). In addition, over the long term, it is said that a school in that space would also have a greenspace/school yard that the community could utilize after school hours. "With so many students able to walk to school, the proposal has 'endless benefits' for the community." – School Committee Representative. It has also been suggested by a neighbour/resident that the municipality could ask for a "re-purchasing option" should the SCDSB decide to sell.

Engineering design can remedy existing drainage and traffic problems with a new opportunity to improve on the existing situation. The Township's Engineer has commented:

The Township is initiating a Transportation (and Trails) Master Plan this year; a part of the study will include transportation planning modeling for 5,10,15 and 20-year horizons through computer software such as Synchro/Visum. The school location and the transportation impact will be inserted into the computer model and this will produce a list of geometry, intersection, and parking lot standards that should be included into the new school site plan design. This approach should eliminate any traffic impact worries or concerns. Residents can provide comments during the public engagement sessions.

With respect to drainage, concerns can be addressed during the detailed design of future parks – additional information is required to comment at this point. Depending on what is proposed and where, the Township may provide a drainage system and/or earth work remediation plan based on the geotechnical report that may be applicable.

The walk to new parks will not be excessive but certainly a change for people. It is most likely that, in no case will anyone's land value have decreased since their purchase.

The parking lot at the existing park is very small and often full. Additional amenities in more locations may help to take the pressure off one location.

Possibilities to Consider include:

191 Maplewood Drive – children's equipment, splashpad, basketball, ? 60 Mike Hart Drive – outdoor pad, basketball, field, splashpad, ? Gold Park Gate – pavilion, picnic tables, adult fitness, trail, dog park, ? 83/84 Greenwood Drive – parking or dog park

FINANCIAL IMPACT

The municipality will gain financially from the sale of parkland and carrying through with the joint venture as proposed with the Simcoe County District School Board.

Manager of Finance Approval:

SUMMARY/OPTIONS

Council may, after careful consideration of all comments:

- 1. Do nothing (in effect, not fulfilling the terms of the agreement with the Simcoe County District School Board), or
- Rezone 152 Greenwood Drive from Open Space Park (OSP) Zone to Institutional (I) Zone to allow for a school use and direct the Manager of Parks and Recreation to begin a public consultation program to collect ideas for parks and for consideration/construction beginning in 2023.

CONCLUSION

At this time, it is recommended that the CAO collect further information to use, to present to Council, to satisfy the comments and/or concerns of the public; and that a further report be presented to Council at a future date.

Respectfully submitted:

Colleen Healey-Dowdall CAO

Attachments: Attachment 1 – Map



PARKS AND OPEN SPACE IN VICINITY OF 152 GREENWOOD DRIVE



- 152 GREENWOOD DRIVE

- 1. 152 GREENWOOD DRIVE
- 2. 191 MAPLEWOOD DRIVE
- 3. OPEN SPACE ON GOLDPARK GATE
- 4. 60 MIKE HART DRIVE

AREA PARKS AND OPEN SPACE